barnson.

Geotechnical Site Investigation Report

41D Robert Hoddle Grove, Bombira NSW

(Our Reference:37522-GR01_A)

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Disclaimer

This report has been prepared solely for Max Walker Constructions in accordance with the scope provided by the client and for the purpose(s) as outlined throughout this report.

Barnson Pty Ltd accepts no liability or responsibility for or in respect of any use or reliance upon this report and its supporting material by anyone other than the client.

Project Name:	41D Robert Hoddle Grove, Bombira NSW
Client:	Max Walker Constructions
Project No.	37522
Report Reference	37522-GR01_A
Date:	23.06.2022
Revision:	A

Prepared by:	Reviewed by:
[redacted]	[redacted]
Gareth Williams Geotechnical Technician	Luke Morris B.E. MIEAust CPEng (NPER) Director

1.0 INTRODUCTION

The following is a report on the geotechnical assessment of a site in accordance with AS1726-2017 “Geotechnical Site Investigations”.

The site investigation was carried out by Barnson Pty Ltd, on behalf of Max Walker Constructions of Mudgee NSW.

Plate 1 – Proposed Development Area

Max Walker Constructions is proposing to construct 15 residential house blocks with associated infrastructure at 41D Robert Hoddle Grove, Bombira NSW. The proposed site features that are covered by this investigation are as follows:

- Proposed House Blocks

The investigation comprised of fifteen (15) boreholes together with field mapping near the site. Details of the field work and laboratory testing are given in the report together with comments relevant to design and construction practice.

1.1 Terminology

The methods used in this report to describe the soil profiles, including visual classification of material types encountered, are in accordance with Australian standard AS1726-2017 “Geotechnical Site Investigations”.

1.2 Limitations

The geotechnical section of Barnson Pty Ltd has conducted this investigation and prepared this report in response to specific instructions from the client to whom this report is addressed. This report is intended for the sole use of the client, and only for the purpose which it is prepared. Any third party who relies on the report or any representation contained in it does so at their own risk.

1.3 Geotechnical Testing

Representative samples from the site were subjected to the following range of tests in accordance with relevant method of Australian Standard AS1289:

- Linear Shrinkage
- PH Determination

NATA reports are attached in ***Appendix D***.

2.0 SITE DESCRIPTION

2.1 General Site Description

The site is situated to the north of Mudgee NSW, in a new residential housing estate area. The surrounding land is reserved for residential purposes.

The site has light scattered grass and weed cover with mature trees scattered over the site.

The overall site is sloping slightly to the northeast and has existing residential houses and roads in the vicinity.

Plate 2 – General view of site facing east.

Plate 3 – General view of site facing North.

Plate 4 – General view of site facing South.

3.0 METHOD OF INVESTIGATION

On the 20th of May 2022, a geotechnical investigation was carried out at the site of the above-mentioned development site. The field drilling was carried out by a geotechnical technician who logged the boreholes on site, who also undertook geological mapping of the nearby area.

A drilling rig with a 90mm auger and tungsten tip was used to excavate fifteen (15) boreholes to a depth of 3.0m within the proposed building areas. These are identified as boreholes 1 through 15.

Disturbed samples (Ds <3kg) were sampled from all relevant boreholes and returned to the Laboratory where Linear Shrinkage testing was conducted on the samples to correlate the material's Shrink Swell index in accordance with AS2870-2011.

Dynamic Cone Penetrometer (DCP) testing was also performed on the site to evaluate the strength and consistency of the natural material present.

3.1 GPS Co-Ordinates

The boreholes were drilled as close as possible to the anticipated location of the proposed structures. GPS Co-ordinates of these were recorded on site to enable plotting of the borehole locations. The following Table 1 shows these co-ordinates.

Table 1: GPS Co-Ordinates of Boreholes

Location	Longitude	Latitude	Proposed Location
Borehole 1	149.609120	-32.578211	Lot 1
Borehole 2	149.608733	-32.578491	Lot 2
Borehole 3	149.608347	-32.578762	Lot 3
Borehole 4	149.605987	-32.578464	Lot 12
Borehole 5	149.605590	-32.578437	Lot 13
Borehole 6	149.604989	-32.578410	Lot 14
Borehole 7	149.604453	-32.578175	Lot 15
Borehole 8	149.604206	-32.577849	Lot 16
Borehole 9	149.604174	-32.577506	Lot 17
Borehole 10	149.605654	-32.577695	Lot 18
Borehole 11	149.606319	-32.577885	Lot 19
Borehole 12	149.606802	-32.578066	Lot 20
Borehole 13	149.608133	-32.578003	Lot 21
Borehole 14	149.607489	-32.578247	Lot 22
Borehole 15	149.606963	-32.578518	Lot 23

The boreholes were recorded on site with a Garmin Oregon 550 handheld GPS, using GDA94 Datum. The co-ordinates have an accuracy of +/- 5m. These locations are also shown on site plan in ***Appendix B***. The borehole logs of sub-surface profiles are attached in ***Appendix C***.

4.0 GENERAL SUB-SURFACE CONDITIONS

4.1 Topsoil

Topsoil was encountered at borehole locations. The topsoil encountered was sandy silt with traces of gravel and was to a depth ranging of 0.2-0.3m.

4.2 Alluvial Soils

Alluvial soils were encountered throughout the boreholes. The alluvial soils generally comprised of wet to slightly moist silts and clays with traces of gravel to depths as shown on the borehole logs. The soils were noted to be of a low to medium plasticity during the field investigation, which was confirmed through subsequent laboratory testing. The borehole logs of sub-surface profiles are attached in ***Appendix C***.

4.3 Regional Geology

Reference to the New South Wales 1:1,000,000 Geological Map indicates the surrounding area consists of *"Shale siltstone, chert, greywacke, greywacke conglomerate, sandstone, arkose, limestone."*.

Rock was not encountered during this investigation.

5.0 NATA LABORATORY TESTING

Disturbed samples were taken during the field investigation. Laboratory testing was carried out on selected samples of all different material types, with details of the sampling and testing shown below:

5.1 Soil Index Properties

Soil Index Properties testing were carried out on samples to aid in classification of the soils encountered and to assist in determining design parameters. This testing included:

5.2 Linear Shrinkage Testing (L.S)

Table 2: Linear Shrinkage Results

Borehole No.	Depth (m)	Proposed Location	Linear Shrinkage (%)
Borehole 1	0.8	Lot 1	7.5
Borehole 1	2.0	Lot 1	8.0
Borehole 2	0.8	Lot 2	2.5
Borehole 2	2.0	Lot 2	2.0
Borehole 3	0.8	Lot 3	5.0
Borehole 3	2.0	Lot 3	4.0
Borehole 4	0.8	Lot 12	8.0
Borehole 4	2.0	Lot 12	11.5
Borehole 5	0.8	Lot 13	10.5
Borehole 5	2.0	Lot 13	12.5
Borehole 6	0.8	Lot 14	9.0
Borehole 6	2.0	Lot 14	13.0
Borehole 7	0.8	Lot 15	6.0
Borehole 7	2.0	Lot 15	9.0
Borehole 8	0.8	Lot 16	13.5
Borehole 8	2.0	Lot 16	12.5

Borehole 9	0.8	Lot 17	6.5
Borehole 9	2.0	Lot 17	12.5
Borehole 10	0.8	Lot 18	16.5
Borehole 10	2.0	Lot 18	12.0
Borehole 11	0.8	Lot 19	10.5
Borehole 11	2.0	Lot 19	10.5
Borehole 12	0.8	Lot 20	12.5
Borehole 12	2.0	Lot 20	15.0
Borehole 13	0.8	Lot 21	16.5
Borehole 13	2.0	Lot 21	9.5
Borehole 14	0.8	Lot 22	16.5
Borehole 14	2.0	Lot 22	14.0
Borehole 15	0.8	Lot 23	15.0
Borehole 15	2.0	Lot 23	11.5

The above test results confirm the material as low to medium plasticity

5.3 Acidic Soils

Acidic ground conditions can be caused by dissolved "aggressive" carbon dioxide, pure and very soft waters, organic and mineral acids and bacterial activity. PH testing was conducted on the site samples to determine if any acidic conditions were present in the soils encountered.

Table 3: PH Testing Results

Borehole No.	Sample Depth (m)	Proposed Location	PH	Exposure Classification
Borehole 1	0.8	Lot 1	6.7	A1
Borehole 2	0.8	Lot 2	6.9	A1
Borehole 3	0.8	Lot 3	6.6	A1
Borehole 4	0.8	Lot 12	7.2	A1
Borehole 5	0.8	Lot 13	6.1	A1
Borehole 6	0.8	Lot 14	7.0	A1
Borehole 7	0.8	Lot 15	6.9	A1
Borehole 8	0.8	Lot 16	7.0	A1
Borehole 9	0.8	Lot 17	7.3	A1
Borehole 10	0.8	Lot 18	7.4	A1
Borehole 11	0.8	Lot 19	7.5	A1
Borehole 12	0.8	Lot 20	7.7	A1
Borehole 13	0.8	Lot 21	8.6	A1
Borehole 14	0.8	Lot 22	9.1	A1
Borehole 15	0.8	Lot 23	8.3	A1

These results show the exposure classification as per Table 5.2 AS2870-2011. Groundwater was not encountered during this investigation.

5.4 Seasonal Surface Movement

From the laboratory test results, as shown attached, an estimated ground surface movement (Ys) was calculated in accordance with AS2870-2011 (using a change in suction at the soil surface $\Delta\mu$ = 1.5pF and a depth of design suction change, Hs = 2.3m) being:

Table 4: Site Classification as per AS2870-2011

Proposed Structure	Site Classification as per AS2870-2011	Ys
Lots 1-3, 12-15, 17, 19	M (Class P as below)	35-40
Lots 16, 18, 20-23	H1 (Class P as below)	50-55

The site is noted to have low bearing soils to a depth of 0.7m-1.4m at the borehole. Clause 2.1.3 of AS2870-2011 states that; *''Sites shall be classified as Class P if the allowable bearing pressure at the foundation level is less than 100kPa"*. This applies to the top 0.7m-1.4m of this site. Due to this extraordinary site feature, it is our opinion that a ***Site Classification of 'P'*** should be adopted for the site in its present condition. The overall seasonal surface movement of the site in its present condition is noted above. The soil reactivity is advised in the above **Table 4**.

5.5 Additional Material Properties

Below are correlated soil properties for the natural soil, which have been determined by correlation with CBR and index tests.

Table 5: Material Properties

Material Strata	Depth (m)	Modulus E_k (MPa)	Undrained Cohesion C_{uk} (kPa)
Firm CLAY/SILT	See borehole logs	10	25
Very Stiff CLAY/SILT	See borehole logs	20	50
Hard CLAY/SILT	See borehole logs	50	120

SITE EARTHWORKS RECOMMENDATIONS

5.6 Excavations

Excavations within the natural silt and clay will be achievable using conventional earthmoving equipment. The civil contractor should be responsible for selecting excavation equipment based on the proposed excavation depths and equipment capabilities.

5.7 General Construction Filling

All earthworks performed on site must be undertaken in a controlled manner, in accordance with a suitable earthwork's specification. Filling should be placed, compacted, inspected and tested in accordance with the Level 2 requirements of AS3798-2007.

5.8 General Bulk Fill Material

All general fill materials used shall be approved clean, hard material, deposited and compacted in the locations specified. Unless notified otherwise, general fill shall be sourced from excavations within the project area. The following conditions should also be satisfied:

- General filling must be compacted to a minimum dry density ratio of 98-100% relative to standard compaction at a moisture content of -2% to +2% of standard optimum moisture content.
- Filling should proceed in layers of 300mm maximum loose thicknesses.
- Layers of filling should be horizontal or benched to suit the surrounding topography.
- The existing subgrade should NOT be used as bulk fill.

5.9 Temporary Batter Slopes

In soil should be graded no steeper than 2 Horizontal (H) in 1 Vertical (V), and protected from erosion by re-directing any surface water flows from the batter face, revegetating etc.

5.10 Permanent Batter Slopes

Batter slopes in clay should be no steeper than 3 Horizontal (H) in 1 Vertical (V) and protected from erosion. Alternatively, fill embankments may be retained with properly designed and constructed retaining walls.

6.0 DESIGN PARAMETERS DISCUSSION

6.1 Single Storey Building Foundations

All foundations for buildings that are similar in size and structure to large residential buildings should be designed with guidance from AS2870-2011 for the site classification provided in section 6.4. No natural soil material shall be used as fill under any proposed buildings. The existing fill layers noted should be compacted and tested to be certified as controlled fill.

6.2 Stiffened Raft/Waffle Slabs

Raft/Waffle slabs should be designed by engineering principles with guidance from AS2870-2011 for the site classification noted. The factored ultimate bearing capacity at depths less than 800mm is Øg Qu = 100kPa. No natural soil material shall be used as fill under any proposed buildings.

6.3 Deep Footings

If deep foundations are required, then bored concrete piers be used and founded onto hard clay. These piers can be designed for a factored ultimate skin friction of 40kPa and a factored ultimate base bearing capacity onto hard silt or clay of 500kPa. The Skin friction should be ignored for 1.5 times the diameter of the pile from the top of the pile due to soil shrinkage. Base bearing capacity assumes pile length is greater than four times the diameter.

Floor slabs should be isolated from any piers. It is recommended the connection be designed to allow vertical movement of the slab to the wall of up to 75mm.

7.0 CONCLUSION

The testing methods adopted are indicative of the site's sub-surface conditions to the depths excavated and to specific sampling and/or testing locations in this investigation, and only at the time the work was carried out. The accuracy of geotechnical engineering advice provided in this report may be limited by unobserved variations in ground conditions across the site in areas between and beyond test locations and by any restrictions in the sampling and testing which was able to be carried out, as well as by the amount of data that could be collected given the project and site constraints. These factors may lead to the possibility that actual ground conditions and materials behaviour observed at the test locations may differ from those which may be encountered elsewhere on the site. If the sub-surface conditions are found to differ from those described in this report, we should be informed immediately to evaluate whether recommendations should be reviewed and amended if necessary.

Appendix A – General Notes

GEOTECHNICAL INVESTIGATION GENERAL NOTES

This report contains the results of a geotechnical investigation conducted for a specific purpose and client. The results should not be used by other parties, or for other purposes, as they may contain neither adequate nor appropriate information. The investigation does not cover contamination issues unless specifically required to do so by the client.

TEST HOLE LOGGING

The information on the test hole logs (boreholes, test pits, exposures etc.) is based on a visual and tactile assessment, except at the discrete locations where the test information is available (field and/or laboratory results). The borehole logs include both factual data and inferred information. Reference should be made to the relevant sheets for the explanation of logging procedures (Soil and Rock Descriptions, Core Log Sheet Notes etc.).

GROUNDWATER

Unless otherwise indicated, the water levels presented on the borehole logs are the levels of free water or seepage in the test hole recorded at the given time of measuring. The actual groundwater level may differ from this recorded level depending on material permeability's (i.e. depending on response time of the measuring instrument). Further, variations of this level could occur with time due to such effects as seasonal, environmental and tidal fluctuations or construction activities. Confirmation of groundwater levels, phreatic surfaces or piezo metric pressures can only be made by appropriate instrumentation techniques and monitoring programmes.

INTERPRETATION OF RESULTS

The discussion or recommendations contained within this report normally are based on a site evaluation from discrete borehole area. Generalised, idealised or inferred subsurface conditions (including any geotechnical cross-sections) have been assumed or prepared by interpolation and/or extrapolation of these data. As such these conditions are an interpretation and must be considered as a guide only.

CHANGE IN CONDITIONS

Local variations or anomalies in the generalised ground conditions do occur in the natural environment, particularly between discrete borehole locations. Additionally, certain design or construction procedures may have been assumed in assessing the soil-structure interaction behaviour of the site. Furthermore, conditions may change at the site from those encountered at the time of the geotechnical investigation through construction activities and constantly changing natural forces.

Any change in design, in construction methods, or in ground conditions as noted during construction, from those assumed or reported should be referred to this firm for appropriate assessment and comment.

GEOTECHNICAL VERIFICATION

Verification of the geotechnical assumptions and/or model is an integral part of the design process – investigation, construction verification and performance monitoring. Variability is a feature of the natural environment and, in many instances, verification of soil or rock quality, or foundation levels are required. There may be a requirement to extend foundation depths to modify a foundation system or to conduct monitoring because of this natural variability. Allowance for verification by geotechnical personnel accordingly should be recognised and programmed during construction.

FOUNDATIONS

Where referred to in the report, the soil or rock quality, or the recommendation depth of any foundation (piles, caissons footings etc.) is an engineering estimate. The estimate is influenced and perhaps limited, by the fieldwork method and testing carried out in connection with the site investigation, and other pertinent information as has been made available. The material quality and/or foundation depth remains, however, an estimate and therefore liable to variation. Foundation drawings, designs and specifications should provide for variations in the final depth, depending upon the ground conditions at each point of support, and allow for geotechnical verification.

REPRODUCTION OF REPORTS

Where it is desired to reproduce the information contained in our geotechnical report, or other technical information, for the inclusion in contract documents or engineering specification of the subject development, such reproductions should include at least all the relevant test hole and test data, together with the appropriate standard description sheets and remarks made in the written report of a factual or descriptive nature.

Reports are the subject of copyright and shall not be reproduced either totally or in part without the express permission of this firm.

ROCK

Rock Strength

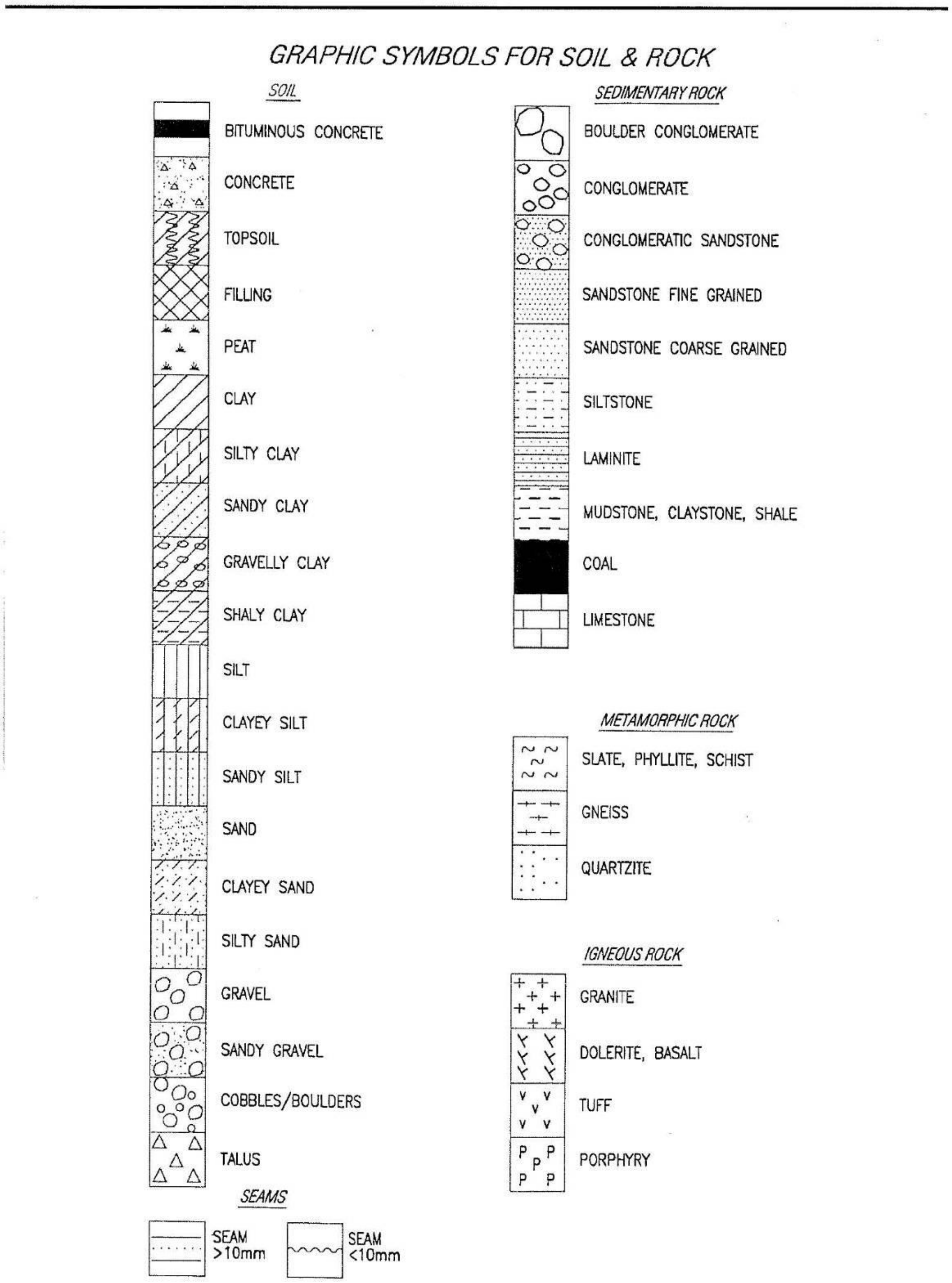
Rock strength is a scale of strength, based on point load index testing, or field testing.

Term	Letter Symbol	Point load index (Mpa) Is(50)	Field guide to strength
Extremely low	EL	< 0.03	Easily remoulded by hand to a material with soil properties.
Very low	VL	0.03 – 0.1	Material crumbles under firm blows with sharp end of pick.
Low	L	0.1 – 0.3	Easily scored by knife, has dull sound under hammer.
Medium	M	0.3 – 1.0	Readily scored with knife, core pieces broken by hand with difficulty
High	H	1 – 3	Rock rings under hammer, core piece broken by pick only.
Very high	VH	3 – 10	Hand specimen breaks with pick after more than one blow.
Extremely high	EH	> 10	Hand specimen breaks with pick after several than one blow.

Rock Weathering

Rock weathering is the degree of rock weathering, determined in the field.

Term	Letter Symbol	Definition
Residual soil	RS	Soil developed on extremely weathered rock.
Extremely weathered rock	XW	Soil is weathered to such an extent that it has soil properties, i.e. it disintegrates or can be remoulded in water.
Distinctly weathered rock	DW	Rock strength usually changed by weathering. The rock may be discoloured, usually by iron staining, porosity is increased.
Slightly weathered rock	SW	Rock is slightly discoloured but shows little or no change of strength from fresh rock.
Fresh rock	FR	Rock shows no sign of decomposition or staining.



GRAPHIC SYMBOLS FOR SOIL & ROCK

SOIL

- BITUMINOUS CONCRETE
- CONCRETE
- TOPSOIL
- FILLING
- PEAT
- CLAY
- SILTY CLAY
- SANDY CLAY
- GRAVELLY CLAY
- SHALY CLAY
- SILT
- CLAYEY SILT
- SANDY SILT
- SAND
- CLAYEY SAND
- SILTY SAND
- GRAVEL
- SANDY GRAVEL
- COBBLES/BOULDERS
- TALUS

SEAMS

- SEAM >10mm
- SEAM <10mm

SEDIMENTARY ROCK

- BOULDER CONGLOMERATE
- CONGLOMERATE
- CONGLOMERATIC SANDSTONE
- SANDSTONE FINE GRAINED
- SANDSTONE COARSE GRAINED
- SILTSTONE
- LAMINITE
- MUDSTONE, CLAYSTONE, SHALE
- COAL
- LIMESTONE

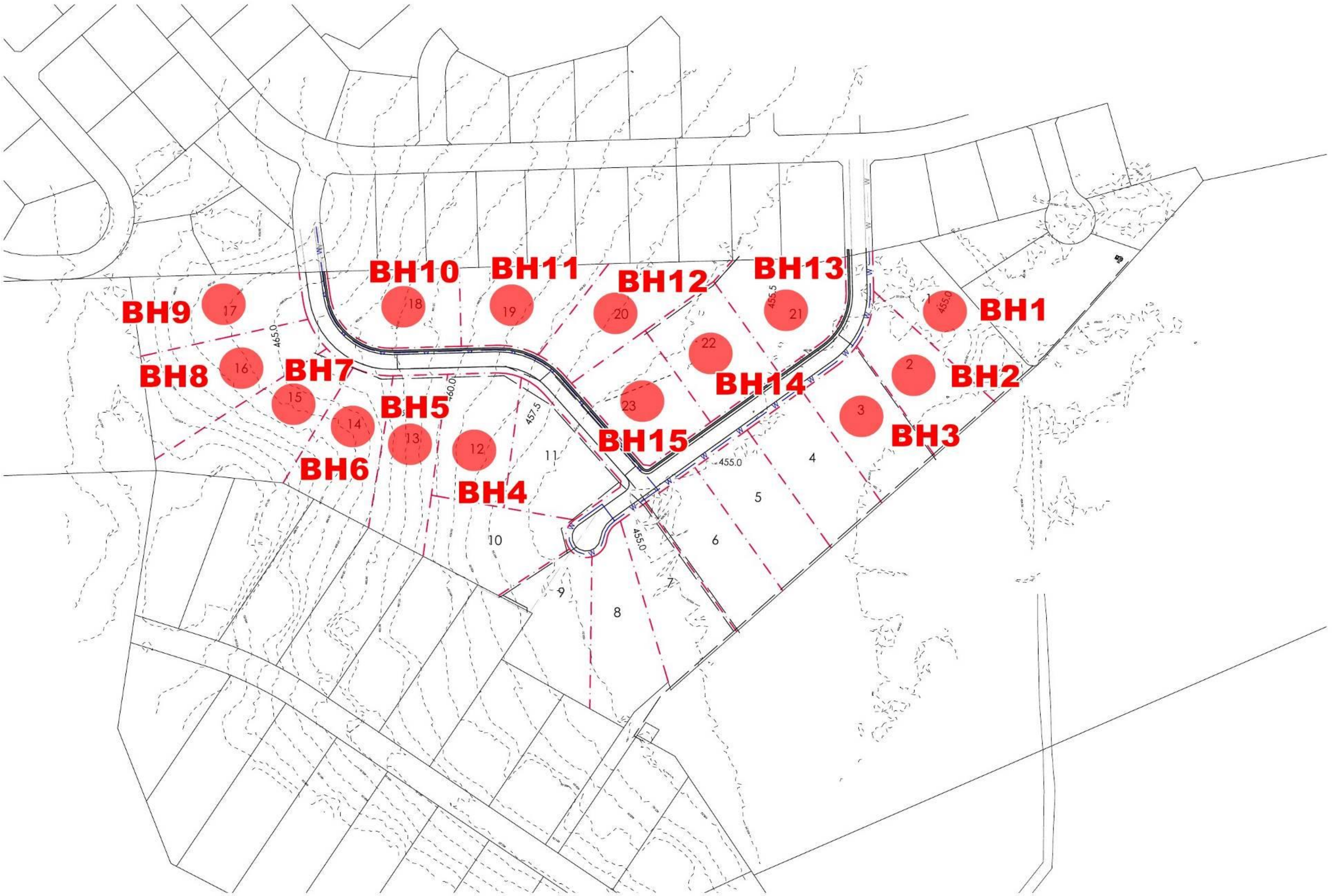
METAMORPHIC ROCK

- SLATE, PHYLLITE, SCHIST
- GNEISS
- QUARTZITE

IGNEOUS ROCK

- GRANITE
- DOLERITE, BASALT
- TUFF
- PORPHYRY

Appendix B – Site Plan with Borehole Locations



PROPOSED SITE PLAN

SCALE 1:1250 AT A1

5m 0 10 20 30 40 50m

SCALE 1:500 AT A1 SHEET | 1:1000 AT A3 SHEET

ISSUED FOR CONSTRUCTION CERTIFICATE	20.09.20	0	J.D.
ISSUED FOR INFORMATION	09.08.19	A	J.D.
AMENDMENTS	DATE	ISSUE	BY

FOR CONSTRUCTION
SUBJECT TO COUNCIL APPROVAL

ARCHITECT

CLIENT
MAX WALKER
MUDGEE NSW 2850

PROJECT
PROPOSED SUBDIVISION
BOMBIRA
MUDGEE NSW 2850

DESIGNED	DRAWN	DATE	SIZE	CAD REF
JO.M.	JO.M.	JULY '19	A1	TX15000.00 - C1.0

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TO BE PRINTED IN COLOUR

DRAWING TITLE
PROPOSED SITE LAYOUT PLAN

PROJECT No.	DRAWING No.	ISSUE
TX14174.00 -	C2.00	0

Appendix C - Borehole Logs

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Barnson
1/36 Darling Street
NSW 2830
Telephone: 1300 BARNSON

BOREHOLE NUMBER 1

PAGE 1 OF 1

CLIENT Max Walker Constructions

PROJECT NAME Site Classification

PROJECT NUMBER 37522

PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22 **COMPLETED** 20/5/22

R.L. SURFACE

DATUM

DRILLING CONTRACTOR Barnson

SLOPE 90°

BEARING ---

EQUIPMENT Scout 1750 Drill Rig

HOLE LOCATION Borehole 1 - Lot 1

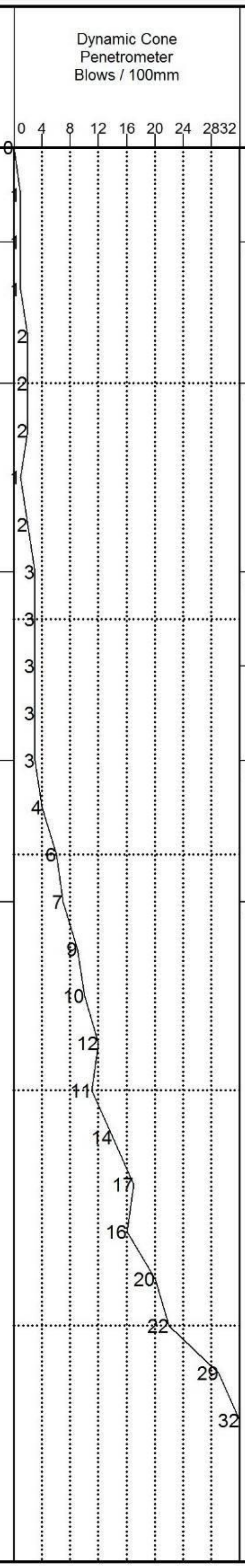
HOLE SIZE 90mm

LOGGED BY HC

CHECKED BY NR

NOTES

Method	Samples	Depth (m)	Classification Symbol	Material Description	Additional Observations
Flight Auger & Tungsten Carbide (T.C) Bit		0.2		Sandy SILT: pale brown	TOPSOIL
		0.5	ML	Clayey SILT: brown: moist: soft to firm: medium plasticity	ALLUVIAL
	Disturbed Sample LS = 7.5%	0.9	CL	Sandy Silty CLAY: brown: moist: soft to firm: medium plasticity	ALLUVIAL
		1.0 1.3	CL	Sandy Silty CLAY: brown: moist: stiff: medium plasticity	ALLUVIAL STANDING WATER LEVEL
		1.5 1.6	SM	Clayey SAND: trace gravel: brown: wet: medium dense to dense: low to medium plasticity	ALLUVIAL
	Disturbed Sample LS = 8.0%	2.0 2.5 3.0	SM	Clayey SAND: trace gravel: brown: wet: very dense: low to medium plasticity	ALLUVIAL



Borehole 1 terminated at 3m

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1/36 Darling Street
NSW 2830
Telephone: 1300 BARNSON

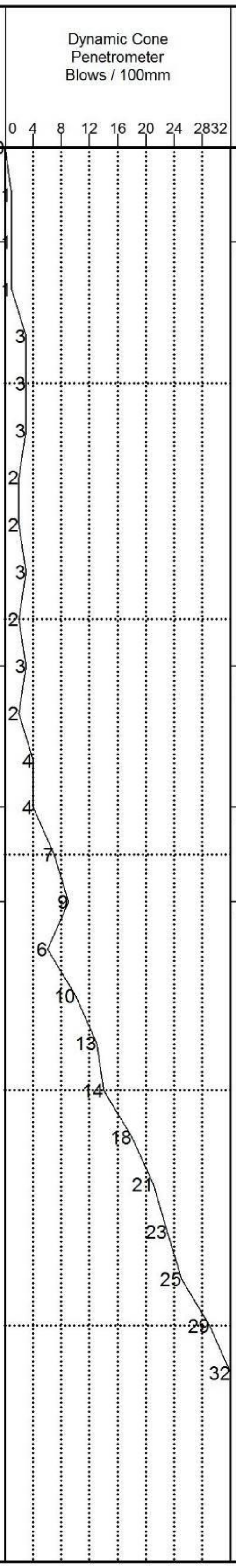
BOREHOLE NUMBER 2

PAGE 1 OF 1

CLIENT Max Walker Constructions
PROJECT NAME Site Classification
PROJECT NUMBER 37522
PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22
COMPLETED 20/5/22
R.L. SURFACE
DATUM
DRILLING CONTRACTOR Barnson
SLOPE 90°
BEARING ---
EQUIPMENT Scout 1750 Drill Rig
HOLE LOCATION Borehole 2 - Lot 2
HOLE SIZE 90mm
LOGGED BY HC
CHECKED BY NR
NOTES

Method	Samples	Depth (m)	Graphic Log	Classification Symbol	Material Description	Additional Observations
Flight Auger & Tungsten Carbide (T.C) Bit		0.2			Sandy SILT: pale brown	TOPSOIL
	Disturbed Sample LS = 2.5%	0.5 1.0 1.1		CL	Sandy CLAY: trace gravel: brown: moist: firm: low plasticity	ALLUVIAL
		1.4		SM	Clayey SAND: trace gravel: brown: wet: loose to medium dense: low plasticity	ALLUVIAL - STANDING WATER LEVEL
		1.5 1.6		SM	Clayey SAND: trace gravel: brown: wet: dense: low plasticity	ALLUVIAL
	Disturbed Sample LS = 2.0%	2.0 2.5 3.0		SM	Clayey SAND: trace gravel: brown: wet: very dense: low to medium plasticity	ALLUVIAL



Borehole 2 terminated at 3m

BOREHOLE / TEST PIT WITH DCP 37522-G01A TO G15A.GPJ GINT STD AUSTRALIA.GDT 8/6/22

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1/36 Darling Street
NSW 2830
Telephone: 1300 BARNSON

BOREHOLE NUMBER 3

PAGE 1 OF 1

CLIENT Max Walker Constructions
PROJECT NAME Site Classification
PROJECT NUMBER 37522
PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22
COMPLETED 20/5/22
R.L. SURFACE
DATUM
DRILLING CONTRACTOR Barnson
SLOPE 90°
BEARING ---
EQUIPMENT Scout 1750 Drill Rig
HOLE LOCATION Borehole 3 - Lot 3
HOLE SIZE 90mm
LOGGED BY HC
CHECKED BY NR
NOTES

Method	Samples	Depth (m)	Graphic Log	Classification Symbol	Material Description	Dynamic Cone Penetrometer Blows / 100mm 0 4 8 12 16 20 24 28 32	Additional Observations
Flight Auger & Tungsten Carbide (T.C) Bit					Sandy SILT: pale brown		TOPSOIL
		0.2		CL	Sandy CLAY: trace gravel: brown: moist: soft to stiff: low to medium plasticity		ALLUVIAL
		0.5					
	Disturbed Sample LS = 5.0%						
		1.0					
		1.2		ML	Clayey Sandy SILT: with gravel: brown: moist: very stiff: low plasticity		ALLUVIAL
		1.5					
	Disturbed Sample LS = 4.0%	2.0					
		2.5					
		3.0					

Borehole 3 terminated at 3m

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1/36 Darling Street
NSW 2830
Telephone: 1300 BARNSON

BOREHOLE NUMBER 4

PAGE 1 OF 1

CLIENT Max Walker Constructions

PROJECT NAME Site Classification

PROJECT NUMBER 37522

PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22 **COMPLETED** 20/5/22

R.L. SURFACE

DATUM

DRILLING CONTRACTOR Barnson

SLOPE 90°

BEARING ---

EQUIPMENT Scout 1750 Drill Rig

HOLE LOCATION Borehole 4 - Lot 12

HOLE SIZE 90mm

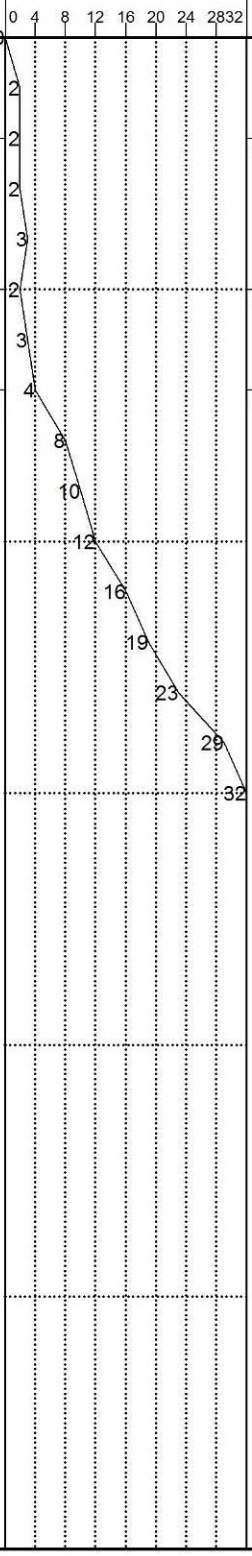
LOGGED BY HC

CHECKED BY NR

NOTES

Method	Samples	Depth (m)	Graphic Log	Classification Symbol	Material Description	Additional Observations
Flight Auger & Tungsten Carbide (T.C) Bit		0.2			Sandy SILT: pale brown	TOPSOIL
		0.5		ML	Sandy SILT: grey-brown: slightly moist: firm: low to medium plasticity	ALLUVIAL
		0.7		CL	Sandy Silty CLAY: brown-orange: slightly moist: firm to stiff: medium plasticity	ALLUVIAL
	Disturbed Sample LS = 8.0% Disturbed Sample LS = 11.5%	1.0 1.5 2.0 2.5 3.0		CL	Sandy Silty CLAY: brown-orange: slightly moist: very stiff to hard: medium plasticity	ALLUVIAL

Dynamic Cone Penetrometer Blows / 100mm



Borehole 4 terminated at 3m

barnson.

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NSW 2830
Telephone: 1300 BARNSON

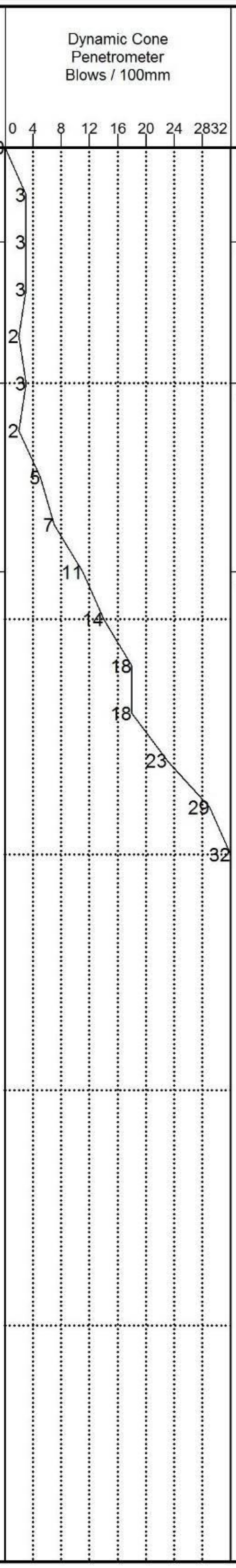
BOREHOLE NUMBER 5

PAGE 1 OF 1

CLIENT Max Walker Constructions
PROJECT NAME Site Classification
PROJECT NUMBER 37522
PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22
COMPLETED 20/5/22
R.L. SURFACE
DATUM
DRILLING CONTRACTOR Barnson
SLOPE 90°
BEARING ---
EQUIPMENT Scout 1750 Drill Rig
HOLE LOCATION Borehole 5 - Lot 13
HOLE SIZE 90mm
LOGGED BY HC
CHECKED BY NR
NOTES

Method	Samples	Depth (m)	Graphic Log	Classification Symbol	Material Description	Additional Observations
Flight Auger & Tungsten Carbide (T.C) Bit		0.2			Sandy SILT: pale brown	TOPSOIL
		0.5		ML	Sandy SILT: grey-brown: slightly moist: firm: low to medium plasticity	ALLUVIAL
	Disturbed Sample LS = 10.5%	0.9		CL	Sandy Silty CLAY: brown-orange: slightly moist: stiff to very stiff: medium plasticity	ALLUVIAL
	Disturbed Sample LS = 12.5%	1.0 1.5 2.0 2.5 3.0		CL	Sandy Silty CLAY: brown-orange: slightly moist: hard: medium to high plasticity	ALLUVIAL



Borehole 5 terminated at 3m

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BOREHOLE NUMBER 6

PAGE 1 OF 1

CLIENT Max Walker Constructions
PROJECT NAME Site Classification
PROJECT NUMBER 37522
PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22
COMPLETED 20/5/22
R.L. SURFACE
DATUM
DRILLING CONTRACTOR Barnson
SLOPE 90°
BEARING ---
EQUIPMENT Scout 1750 Drill Rig
HOLE LOCATION Borehole 6 - Lot 14
HOLE SIZE 90mm
LOGGED BY HC
CHECKED BY NR
NOTES

Method	Samples	Depth (m)	Graphic Log	Classification Symbol	Material Description	Dynamic Cone Penetrometer Blows / 100mm 0 4 8 12 16 20 24 28 32	Additional Observations
Flight Auger & Tungsten Carbide (T.C) Bit					Sandy SILT: dark brown		TOPSOIL
		0.2		ML	Clayey SILT: brown: slightly moist: soft to stiff: low to medium plasticity		ALLUVIAL
		0.5					
		0.7		CL	Sandy Silty CLAY: brown-yellow: slightly moist: very stiff: medium to high plasticity		ALLUVIAL
	Disturbed Sample LS = 9.0%						
		1.0					
		1.5					
	Disturbed Sample LS = 13.0%	2.0					
		2.5					
		3.0					

Borehole 6 terminated at 3m

BOREHOLE NUMBER 7

PAGE 1 OF 1

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CLIENT Max Walker Constructions
PROJECT NAME Site Classification
PROJECT NUMBER 37522
PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22
COMPLETED 20/5/22
R.L. SURFACE
DATUM
DRILLING CONTRACTOR Barnson
SLOPE 90°
BEARING ---
EQUIPMENT Scout 1750 Drill Rig
HOLE LOCATION Borehole 7 - Lot 15
HOLE SIZE 90mm
LOGGED BY HC
CHECKED BY NR
NOTES

Method	Samples	Depth (m)	Graphic Log	Classification Symbol	Material Description	Dynamic Cone Penetrometer Blows / 100mm 0 4 8 12 16 20 24 28 32	Additional Observations
Flight Auger & Tungsten Carbide (T.C) Bit			░		Sandy SILT: pale brown		TOPSOIL
		0.2	▥	ML	Sandy SILT: grey-brown: slightly moist: soft to stiff: low to medium plasticity		ALLUVIAL
		0.5	▨	CL	Sandy Silty CLAY: brown-orange: slightly moist: very stiff: medium plasticity		ALLUVIAL
	Disturbed Sample LS = 6.0%		▨				
		1.0	▨				
		1.5	▨				
	Disturbed Sample LS = 9.0%	2.0	▨				
		2.5	▨				
		3.0	▨				

Borehole 7 terminated at 3m

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BOREHOLE NUMBER 8

PAGE 1 OF 1

CLIENT Max Walker Constructions

PROJECT NAME Site Classification

PROJECT NUMBER 37522

PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22 **COMPLETED** 20/5/22

R.L. SURFACE

DATUM

DRILLING CONTRACTOR Barnson

SLOPE 90°

BEARING ---

EQUIPMENT Scout 1750 Drill Rig

HOLE LOCATION Borehole 8 - Lot 16

HOLE SIZE 90mm

LOGGED BY HC

CHECKED BY NR

NOTES

Method	Samples	Depth (m)	Graphic Log	Classification Symbol	Material Description	Dynamic Cone Penetrometer Blows / 100mm 0 4 8 12 16 20 24 28 32	Additional Observations
Flight Auger & Tungsten Carbide (T.C) Bit		0.2			Sandy SILT: brown		TOPSOIL
		0.5 0.6		ML	Clayey SILT: brown-red: moist: soft to stiff: low plasticity		ALLUVIAL
	Disturbed Sample LS = 13.5%	1.0 1.5 1.8		CL	Sandy Silty CLAY: brown-red: slightly moist: very stiff: medium to high plasticity		ALLUVIAL
	Disturbed Sample LS = 12.5%	2.0 2.5 3.0		CL	Sandy Silty CLAY: brown-yellow: slightly moist: hard: medium to high plasticity		ALLUVIAL

Borehole 8 terminated at 3m

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BOREHOLE NUMBER 9

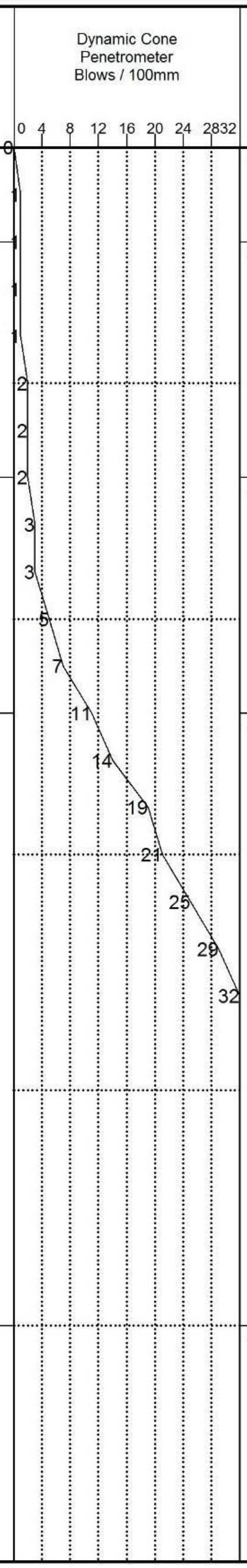
PAGE 1 OF 1

CLIENT Max Walker Constructions
PROJECT NAME Site Classification
PROJECT NUMBER 37522
PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22
COMPLETED 20/5/22
R.L. SURFACE
DATUM
DRILLING CONTRACTOR Barnson
SLOPE 90°
BEARING ---
EQUIPMENT Scout 1750 Drill Rig
HOLE LOCATION Borehole 9 - Lot 17
HOLE SIZE 90mm
LOGGED BY HC
CHECKED BY NR
NOTES

Method: Flight Auger & Tungsten Carbide (T.C) Bit

Samples	Depth (m)	Classification Symbol	Material Description	Additional Observations
	0.0 – 0.2		Sandy SILT: brown	TOPSOIL
	0.2 – 0.7	ML	Clayey SILT: pale brown: moist: soft to firm: low plasticity	ALLUVIAL
Disturbed Sample LS = 6.5%	0.7 – 1.2	CL	Sandy Silty CLAY: brown: slightly moist: stiff to very stiff: low to medium plasticity	ALLUVIAL
Disturbed Sample LS = 12.5%	1.2 – 2.5	CL	Sandy Silty CLAY: brown: slightly moist: hard: medium to high plasticity	ALLUVIAL
	2.5 – 3.0	CL	Sandy Silty CLAY: brown-yellow: wet: hard: medium to high plasticity	ALLUVIAL


Dynamic Cone Penetrometer Blows / 100mm

Borehole 9 terminated at 3m

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BOREHOLE NUMBER 10

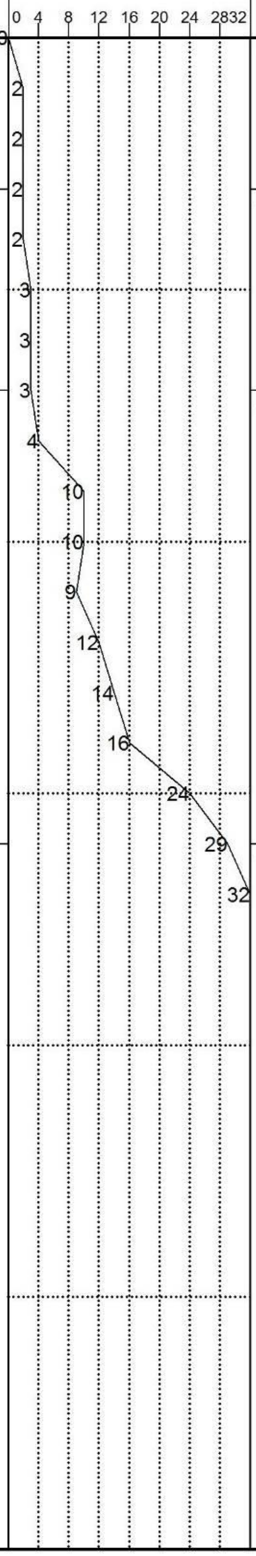
PAGE 1 OF 1

CLIENT Max Walker Constructions
PROJECT NAME Site Classification
PROJECT NUMBER 37522
PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22
COMPLETED 20/5/22
R.L. SURFACE
DATUM
DRILLING CONTRACTOR Barnson
SLOPE 90°
BEARING ---
EQUIPMENT Scout 1750 Drill Rig
HOLE LOCATION Borehole 10 - Lot 18
HOLE SIZE 90mm
LOGGED BY HC
CHECKED BY NR
NOTES

Method	Samples	Depth (m)	Graphic Log	Classification Symbol	Material Description	Additional Observations
Flight Auger & Tungsten Carbide (T.C) Bit					Sandy SILT: trace gravel: brown	TOPSOIL
		0.3		ML	Clayey SILT: brown: moist: firm to stiff: medium plasticity	ALLUVIAL
		0.5				
	Disturbed Sample LS = 16.5%	0.7		CH	Silty CLAY: brown mottled red-yellow: slightly moist: stiff to hard: high plasticity	ALLUVIAL
		1.0				
		1.5				
		1.6		CL	Silty CLAY: brown-red: slighlty moist: hard: medium to high plasticity	ALLUVIAL
	Disturbed Sample LS = 12.0%	2.0				
		2.5				
		3.0				

Dynamic Cone Penetrometer Blows / 100mm



Borehole 10 terminated at 3m

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BOREHOLE NUMBER 11

PAGE 1 OF 1

CLIENT Max Walker Constructions
PROJECT NAME Site Classification
PROJECT NUMBER 37522
PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22
COMPLETED 20/5/22
R.L. SURFACE
DATUM
DRILLING CONTRACTOR Barnson
SLOPE 90°
BEARING ---
EQUIPMENT Scout 1750 Drill Rig
HOLE LOCATION Borehole 11 - Lot 19
HOLE SIZE 90mm
LOGGED BY HC
CHECKED BY NR
NOTES

Method	Samples	Depth (m)	Graphic Log	Classification Symbol	Material Description	Dynamic Cone Penetrometer Blows / 100mm 0 4 8 12 16 20 24 2832	Additional Observations
Flight Auger & Tungsten Carbide (T.C) Bit					Sandy SILT: pale brown		TOPSOIL
		0.2		ML	Sandy SILT: grey-brown: slightly moist: soft to stiff: low to medium plasticity		ALLUVIAL
		0.5	▨	CL	Sandy Silty CLAY: brown-orange: slightly moist: very stiff: medium plasticity		ALLUVIAL
	Disturbed Sample LS = 10.5%						
		1.0					
		1.5					
	Disturbed Sample LS = 10.5%	2.0					
		2.5					
		3.0					

Borehole 11 terminated at 3m

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BOREHOLE NUMBER 12

PAGE 1 OF 1

CLIENT Max Walker Constructions
PROJECT NAME Site Classification
PROJECT NUMBER 37522
PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22
COMPLETED 20/5/22
R.L. SURFACE
DATUM
DRILLING CONTRACTOR Barnson
SLOPE 90°
BEARING ---
EQUIPMENT Scout 1750 Drill Rig
HOLE LOCATION Borehole 12 - Lot 20
HOLE SIZE 90mm
LOGGED BY HC
CHECKED BY NR
NOTES

Method	Samples	Depth (m)	Graphic Log	Classification Symbol	Material Description	Dynamic Cone Penetrometer Blows / 100mm 0 4 8 12 16 20 24 2832	Additional Observations
Flight Auger & Tungsten Carbide (T.C) Bit		0.2			Sandy SILT: brown		TOPSOIL
		0.5 0.7		ML	Clayey SILT: pale brown: moist: soft to firm: low plasticity		ALLUVIAL
	Disturbed Sample LS = 12.5%	1.0 1.5		CH	Sandy Silty CLAY: brown: slightly moist: stiff to very stiff: medium to high plasticity		ALLUVIAL
	Disturbed Sample LS = 15.0%	2.0 2.5					
		3.0		CH	Sandy Silty CLAY: brown-yellow: wet: hard: high plasticity		ALLUVIAL

Borehole 12 terminated at 3m

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BOREHOLE NUMBER 13

PAGE 1 OF 1

CLIENT Max Walker Constructions

PROJECT NAME Site Classification

PROJECT NUMBER 37522

PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22 **COMPLETED** 20/5/22

R.L. SURFACE

DATUM

DRILLING CONTRACTOR Barnson

SLOPE 90°

BEARING ---

EQUIPMENT Scout 1750 Drill Rig

HOLE LOCATION Borehole 13 - Lot 21

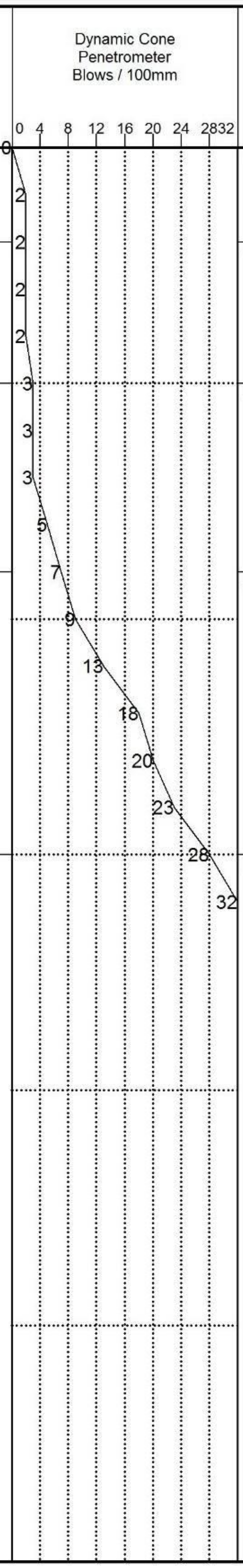
HOLE SIZE 90mm

LOGGED BY HC

CHECKED BY NR

NOTES

Method	Samples	Depth (m)	Classification Symbol	Material Description	Additional Observations
Flight Auger & Tungsten Carbide (T.C) Bit		0.0–0.2		Sandy SILT: dark brown	TOPSOIL
		0.2–0.5	ML	Clayey SILT: grey: moist: firm to stiff: low to medium plasticity	ALLUVIAL
	Disturbed Sample LS = 16.5%	0.5–0.9	CH	Sandy Silty CLAY: brown: slightly moist: very stiff: high plasticity	ALLUVIAL
		0.9–1.5	CH	Sandy Silty CLAY: brown: slightly moist: hard: medium to high plasticity	ALLUVIAL
	Disturbed Sample LS = 9.5%	1.5–3.0	CL	Sandy CLAY: trace gravel: brown: slighlty moist: hard: medium plasticity	ALLUVIAL



Borehole 13 terminated at 3m

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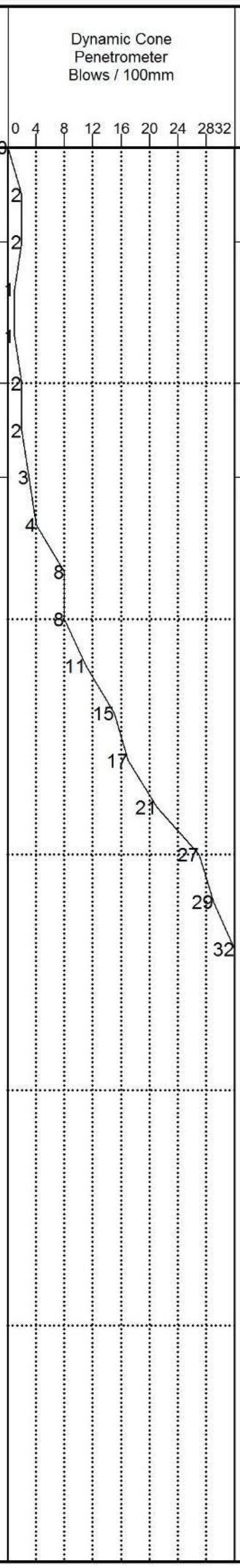
BOREHOLE NUMBER 14

CLIENT Max Walker Constructions
PROJECT NAME Site Classification
PROJECT NUMBER 37522
PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22
COMPLETED 20/5/22
R.L. SURFACE
DATUM
DRILLING CONTRACTOR Barnson
SLOPE 90°
BEARING ---
EQUIPMENT Scout 1750 Drill Rig
HOLE LOCATION Borehole 14 - Lot 22
HOLE SIZE 90mm
LOGGED BY HC
CHECKED BY NR
NOTES

Method	Samples	Depth (m)	Classification Symbol	Material Description	Additional Observations
Flight Auger & Tungsten Carbide (T.C) Bit		0.0 – 0.2		Sandy SILT: dark brown	TOPSOIL
		0.2 – 0.5	ML	Clayey SILT: brown: slightly moist: soft to firm: low to medium plasticity	ALLUVIAL
		0.5 – 0.7	ML	Clayey SILT: brown: slightly moist: firm to stiff: low to medium plasticity	ALLUVIAL
	Disturbed Sample LS = 16.5% (0.7 – 0.9) Disturbed Sample LS = 14.0% (1.9 – 2.1)	0.7 – 3.0	CH	Sandy Silty CLAY: brown-yellow: slightly moist: very stiff to hard: medium to high plasticity	ALLUVIAL


Dynamic Cone Penetrometer Blows / 100mm

Borehole 14 terminated at 3m

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NSW 2830
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BOREHOLE NUMBER 15

PAGE 1 OF 1

CLIENT Max Walker Constructions

PROJECT NAME Site Classification

PROJECT NUMBER 37522

PROJECT LOCATION 41D Robert Hoddle Grove, Bombira NSW

DATE STARTED 20/5/22 **COMPLETED** 20/5/22

R.L. SURFACE

DATUM

DRILLING CONTRACTOR Barnson

SLOPE 90°

BEARING ---

EQUIPMENT Scout 1750 Drill Rig

HOLE LOCATION Borehole 15 - Lot 23

HOLE SIZE 90mm

LOGGED BY HC

CHECKED BY NR

NOTES

Method	Samples	Depth (m)	Graphic Log	Classification Symbol	Material Description	Dynamic Cone Penetrometer Blows / 100mm 0 4 8 12 16 20 24 28 32	Additional Observations
Flight Auger & Tungsten Carbide (T.C) Bit					Sandy SILT: brown		TOPSOIL
		0.2		ML	Clayey SILT: pale brown: moist: soft to firm: low plasticity		ALLUVIAL
		0.5					
	Disturbed Sample LS = 15.0%	0.7		CH	Sandy Silty CLAY: brown: slightly moist: stiff to very stiff: medium to high plasticity		ALLUVIAL
		1.0					
		1.5					
	Disturbed Sample LS = 11.5%	2.0					
		2.5		CL	Sandy Silty CLAY: brown-yellow: wet: hard: medium plasticity		ALLUVIAL
		3.0					

Borehole 15 terminated at 3m

Appendix D - NATA Laboratory Reports

Material Test Report

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Barnson Pty Ltd
Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

Accredited for compliance with ISO/IEC 17025 - Testing

NATA
WORLD RECOGNISED
ACCREDITATION

[redacted]

Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270A
Date Sampled:	20/05/2022
Dates Tested:	20/05/2022 - 08/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 1 - Lot 1 Nelthorpe Street Lat: -32.578211, Long: 149.609120, Depth: 800mm**
Lot No:	**1**
Material:	Brown Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	7.5		
Cracking Crumbling Curling	None		

Material Test Report

Barnson Pty Ltd
Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
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Email: jeremy@barnson.com.au

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[redacted]

Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270B
Date Sampled:	20/05/2022
Dates Tested:	20/05/2022 - 08/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 1 - Lot 1 Nelthorpe Street Lat: -32.578211, Long: 149.609120, Depth: 2.0m**
Lot No:	**1**
Material:	Brown Clayey SAND Trace Gravel

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	8.0		
Cracking Crumbling Curling	None		

Material Test Report

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Dubbo Laboratory
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[redacted]

Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician

NATA Accredited Laboratory Number: 9605

Report Number: **37522-3**
Issue Number: 1
Date Issued: 08/06/2022
Client: Max Walker Constructions
P.O. Box 1151, Mudgee NSW 2850
Contact: Colleen Walker
Project Number: 37522
Project Name: Subdivision Developement
Project Location: 41D Robert Hoddle Grove, Bombira NSW
Work Request: 6270
Sample Number: D22-6270C
Date Sampled: 20/05/2022
Dates Tested: 20/05/2022 - 08/06/2022
Sampling Method: AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection: Selected by Client
Sample Location: **Borehole 2 - Lot 2 Nelthorpe Street Lat: -32.578491, Long: 149.608733, Depth: 800mm**
Lot No: **2**
Material: Brown Sandy CLAY Trace Gravel

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	2.5		
Cracking Crumbling Curling	None		

Material Test Report

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Dubbo Laboratory
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Email: jeremy@barnson.com.au

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Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270D
Date Sampled:	20/05/2022
Dates Tested:	20/05/2022 - 08/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 2 - Lot 2 Nelthorpe Street Lat: -32.578491, Long: 149.608733, Depth: 2.0m**
Lot No:	**2**
Material:	Brown Clayey SAND Trace Gravel

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	2.0		
Cracking Crumbling Curling	None		

Material Test Report

Barnson Pty Ltd
Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
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Email: jeremy@barnson.com.au

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Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270E
Date Sampled:	20/05/2022
Dates Tested:	20/05/2022 - 07/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 3 - Lot 3 Nelthorpe Street Lat: -32.578762, Long: 149.608347, Depth: 800mm**
Lot No:	**3**
Material:	Brown Sandy CLAY Trace Gravel

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	5.0		
Cracking Crumbling Curling	None		

Material Test Report

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Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number: **37522-3**
Issue Number: 1
Date Issued: 08/06/2022
Client: Max Walker Constructions
P.O. Box 1151, Mudgee NSW 2850
Contact: Colleen Walker
Project Number: 37522
Project Name: Subdivision Developement
Project Location: 41D Robert Hoddle Grove, Bombira NSW
Work Request: 6270
Sample Number: D22-6270F
Date Sampled: 20/05/2022
Dates Tested: 20/05/2022 - 07/06/2022
Sampling Method: AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection: Selected by Client
Sample Location: **Borehole 3 - Lot 3 Nelthorpe Street Lat: -32.578762, Long: 149.608347, Depth: 2.0m**
Lot No: **3**
Material: Brown Clayey Sandy SILT With Gravel

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	4.0		
Cracking Crumbling Curling	None		

Material Test Report

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270G
Date Sampled:	20/05/2022
Dates Tested:	20/05/2022 - 08/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 4 - Lot 12 Page Street Lat: -32.578464, Long: 149.605987, Depth: 800mm**
Lot No:	**12**
Material:	Brown-Orange Sandy Silty CLAY

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16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
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Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician

NATA Accredited Laboratory Number: 9605

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	8.0		
Cracking Crumbling Curling	None		

Material Test Report

Report Number: **37522-3**
Issue Number: 1
Date Issued: 08/06/2022
Client: Max Walker Constructions
P.O. Box 1151, Mudgee NSW 2850
Contact: Colleen Walker
Project Number: 37522
Project Name: Subdivision Developement
Project Location: 41D Robert Hoddle Grove, Bombira NSW
Work Request: 6270
Sample Number: D22-6270H
Date Sampled: 20/05/2022
Dates Tested: 20/05/2022 - 08/06/2022
Sampling Method: AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection: Selected by Client
Sample Location: **Borehole 4 - Lot 12 Page Street Lat: -32.578464, Long: 149.605987, Depth: 2.0m**
Lot No: **12**
Material: Brown-Orange Sandy Silty CLAY

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Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	11.5		
Cracking Crumbling Curling	Curling		

Material Test Report

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270I
Date Sampled:	20/05/2022
Dates Tested:	20/05/2022 - 08/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 5 - Lot 13 Page Street Lat: -32.578437, Long: 149.605590, Depth: 800mm**
Lot No:	**13**
Material:	Brown-Orange Sandy Silty CLAY

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Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	10.5		
Cracking Crumbling Curling	None		

Material Test Report

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Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number: **37522-3**
Issue Number: 1
Date Issued: 08/06/2022
Client: Max Walker Constructions
P.O. Box 1151, Mudgee NSW 2850
Contact: Colleen Walker
Project Number: 37522
Project Name: Subdivision Developement
Project Location: 41D Robert Hoddle Grove, Bombira NSW
Work Request: 6270
Sample Number: D22-6270J
Date Sampled: 20/05/2022
Dates Tested: 20/05/2022 - 08/06/2022
Sampling Method: AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection: Selected by Client
Sample Location: **Borehole 5 - Lot 13 Page Street Lat: -32.578437, Long: 149.605590, Depth: 2.0m**
Lot No: **13**
Material: Brown-Orange Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	12.5		
Cracking Crumbling Curling	Curling		

Material Test Report

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16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270K
Date Sampled:	20/05/2022
Dates Tested:	20/05/2022 - 07/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 6 - Lot 14 Page Street Lat: -32.578410, Long: 149.604989, Depth: 800mm**
Lot No:	**14**
Material:	Brown-Yellow Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	9.0		
Cracking Crumbling Curling	Curling		

Material Test Report

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270L
Date Sampled:	20/05/2022
Dates Tested:	20/05/2022 - 07/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 6 - Lot 14 Page Street Lat: -32.578410, Long: 149.604989, Depth: 2.0m**
Lot No:	**14**
Material:	Brown-Yellow Sandy Silty CLAY

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Barnson Pty Ltd
Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Geotechnical Technician

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Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	13.0		
Cracking Crumbling Curling	Curling		

Material Test Report

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Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician

NATA Accredited Laboratory Number: 9605

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270M
Date Sampled:	20/05/2022
Dates Tested:	20/05/2022 - 08/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 7 - Lot 15 Page Street Lat: -32.578175, Long: 149.604453, Depth: 800mm**
Lot No:	**15**
Material:	Brown-Orange Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	6.0		
Cracking Crumbling Curling	None		

Material Test Report

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Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Geotechnical Technician

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Report Number: **37522-3**
Issue Number: 1
Date Issued: 08/06/2022
Client: Max Walker Constructions
P.O. Box 1151, Mudgee NSW 2850
Contact: Colleen Walker
Project Number: 37522
Project Name: Subdivision Developement
Project Location: 41D Robert Hoddle Grove, Bombira NSW
Work Request: 6270
Sample Number: D22-6270N
Date Sampled: 20/05/2022
Dates Tested: 20/05/2022 - 08/06/2022
Sampling Method: AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection: Selected by Client
Sample Location: **Borehole 7 - Lot 15 Page Street Lat: -32.578175, Long: 149.604453, Depth: 2.0m**
Lot No: **15**
Material: Brown-Orange Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	9.0		
Cracking Crumbling Curling	None		

Material Test Report

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Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number: **37522-3**
Issue Number: 1
Date Issued: 08/06/2022
Client: Max Walker Constructions
P.O. Box 1151, Mudgee NSW 2850
Contact: Colleen Walker
Project Number: 37522
Project Name: Subdivision Developement
Project Location: 41D Robert Hoddle Grove, Bombira NSW
Work Request: 6270
Sample Number: D22-6270O
Date Sampled: 20/05/2022
Dates Tested: 20/05/2022 - 07/06/2022
Sampling Method: AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection: Selected by Client
Sample Location: **Borehole 8 - Lot 16 Page Street Lat: -32.577849, Long: 149.604206, Depth: 800mm**
Lot No: **16**
Material: Brown-Red Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	13.5		
Cracking Crumbling Curling	Curling		

Material Test Report

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16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number: **37522-3**
Issue Number: 1
Date Issued: 08/06/2022
Client: Max Walker Constructions
P.O. Box 1151, Mudgee NSW 2850
Contact: Colleen Walker
Project Number: 37522
Project Name: Subdivision Developement
Project Location: 41D Robert Hoddle Grove, Bombira NSW
Work Request: 6270
Sample Number: D22-6270P
Date Sampled: 20/05/2022
Dates Tested: 20/05/2022 - 08/06/2022
Sampling Method: AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection: Selected by Client
Sample Location: **Borehole 8 - Lot 16 Page Street Lat: -32.577849, Long: 149.604206, Depth: 2.0m**
Lot No: **16**
Material: Brown-Yellow Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	12.5		
Cracking Crumbling Curling	Curling		

Material Test Report

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Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270Q
Date Sampled:	20/05/2022
Dates Tested:	20/05/2022 - 07/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 9 - Lot 17 Page Street Lat: -32.577506, Long: 149.604174, Depth: 800mm**
Lot No:	**17**
Material:	Brown Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	6.5		
Cracking Crumbling Curling	Cracking		

Material Test Report

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Barnson Pty Ltd
Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270R
Date Sampled:	20/05/2022
Dates Tested:	20/05/2022 - 08/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 9 - Lot 17 Page Street Lat: -32.577506, Long: 149.604174, Depth: 2.0m**
Lot No:	**17**
Material:	Brown Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	12.5		
Cracking Crumbling Curling	Curling		

Material Test Report

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Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au
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Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270S
Date Sampled:	23/05/2022
Dates Tested:	23/05/2022 - 07/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 10 - Lot 18 Page Street Lat: -32.577695, Long: 149.605654, Depth: 800mm**
Lot No:	**18**
Material:	Brown Mottled Red-Yellow Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	16.5		
Cracking Crumbling Curling	Curling		

Material Test Report

Report Number: **37522-3**
Issue Number: 1
Date Issued: 08/06/2022
Client: Max Walker Constructions
P.O. Box 1151, Mudgee NSW 2850
Contact: Colleen Walker
Project Number: 37522
Project Name: Subdivision Developement
Project Location: 41D Robert Hoddle Grove, Bombira NSW
Work Request: 6270
Sample Number: D22-6270T
Date Sampled: 23/05/2022
Dates Tested: 23/05/2022 - 07/06/2022
Sampling Method: AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection: Selected by Client
Sample Location: **Borehole 10 - Lot 18 Page Street Lat: -32.577695, Long: 149.605654, Depth: 2.0m**
Lot No: **18**
Material: Brown-Red Silty CLAY

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Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Geotechnical Technician

NATA Accredited Laboratory Number: 9605

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	12.0		
Cracking Crumbling Curling	None		

Material Test Report

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270U
Date Sampled:	23/05/2022
Dates Tested:	23/05/2022 - 08/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 11 - Lot 19 Page Street E: -32.577885, N: 149.606319, Depth: 800mm**
Lot No:	**19**
Material:	Brown-Orange Sandy Silty CLAY

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Barnson Pty Ltd
Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	10.5		
Cracking Crumbling Curling	None		

Material Test Report

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Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au
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Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number: **37522-3**
Issue Number: 1
Date Issued: 08/06/2022
Client: Max Walker Constructions
P.O. Box 1151, Mudgee NSW 2850
Contact: Colleen Walker
Project Number: 37522
Project Name: Subdivision Developement
Project Location: 41D Robert Hoddle Grove, Bombira NSW
Work Request: 6270
Sample Number: D22-6270V
Date Sampled: 23/05/2022
Dates Tested: 23/05/2022 - 08/06/2022
Sampling Method: AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection: Selected by Client
Sample Location: **Borehole 11 - Lot 19 Page Street Lat: -32.577885, Long: 149.606319, Depth: 2.0m**
Lot No: **19**
Material: Brown-Orange Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	10.5		
Cracking Crumbling Curling	Curling		

Material Test Report

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Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Geotechnical Technician

NATA Accredited Laboratory Number: 9605

Report Number: **37522-3**
Issue Number: 1
Date Issued: 08/06/2022
Client: Max Walker Constructions
P.O. Box 1151, Mudgee NSW 2850
Contact: Colleen Walker
Project Number: 37522
Project Name: Subdivision Developement
Project Location: 41D Robert Hoddle Grove, Bombira NSW
Work Request: 6270
Sample Number: D22-6270W
Date Sampled: 23/05/2022
Dates Tested: 23/05/2022 - 08/06/2022
Sampling Method: AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection: Selected by Client
Sample Location: **Borehole 12 - Lot 20 Page Street Lat: -32.578066, Long: 149.606802, Depth: 800mm**
Lot No: **20**
Material: Brown Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	12.5		
Cracking Crumbling Curling	Curling		

Material Test Report

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270X
Date Sampled:	23/05/2022
Dates Tested:	23/05/2022 - 08/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 12 - Lot 20 Page Street Lat: -32.578066, Long: 149.606802, Depth: 2.0m**
Lot No:	**20**
Material:	Brown Sandy Silty CLAY

Barnson Pty Ltd
Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	15.0		
Cracking Crumbling Curling	Curling		

Material Test Report

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Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au
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Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270Y
Date Sampled:	23/05/2022
Dates Tested:	23/05/2022 - 08/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 13 - Lot 21 Nelthorpe Street Lat: -32.578003, Long: 149.608133, Depth: 800mm**
Lot No:	**21**
Material:	Brown Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	16.5		
Cracking Crumbling Curling	Curling		

Material Test Report

Barnson Pty Ltd
Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au
Accredited for compliance with ISO/IEC 17025 - Testing

[redacted]

Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270Z
Date Sampled:	23/05/2022
Dates Tested:	23/05/2022 - 08/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 13 - Lot 21 Nelthorpe Street Lat: -32.578003, Long: 149.608133, Depth: 2.0m**
Lot No:	**21**
Material:	Brown Sandy CLAY Trace Gravel

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	9.5		
Cracking Crumbling Curling	None		

Material Test Report

Report Number:	**37522-3**
Issue Number:	1
Date Issued:	08/06/2022
Client:	Max Walker Constructions P.O. Box 1151, Mudgee NSW 2850
Contact:	Colleen Walker
Project Number:	37522
Project Name:	Subdivision Developement
Project Location:	41D Robert Hoddle Grove, Bombira NSW
Work Request:	6270
Sample Number:	D22-6270AA
Date Sampled:	23/05/2022
Dates Tested:	23/05/2022 - 07/06/2022
Sampling Method:	AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection:	Selected by Client
Sample Location:	**Borehole 14 - Lot 22 Nelthorpe Street Lat: -32.578247, Long: 149.607489, Depth: 800mm**
Lot No:	**22**
Material:	Brown-Yellow Sandy Silty CLAY

barnson.
Barnson Pty Ltd
Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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NATA
WORLD RECOGNISED ACCREDITATION

[redacted]

Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician

NATA Accredited Laboratory Number: 9605

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	16.5		
Cracking Crumbling Curling	Curling		

Material Test Report

barnson.
Barnson Pty Ltd
Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

Accredited for compliance with ISO/IEC 17025 - Testing

[redacted]

Jeremy Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number: **37522-3**
Issue Number: 1
Date Issued: 08/06/2022
Client: Max Walker Constructions
P.O. Box 1151, Mudgee NSW 2850
Contact: Colleen Walker
Project Number: 37522
Project Name: Subdivision Developement
Project Location: 41D Robert Hoddle Grove, Bombira NSW
Work Request: 6270
Sample Number: D22-6270AB
Date Sampled: 23/05/2022
Dates Tested: 23/05/2022 - 07/06/2022
Sampling Method: AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection: Selected by Client
Sample Location: **Borehole 14 - Lot 22 Nelthorpe Street Lat: -32.578247, Long: 149.607489, Depth: 2.0m**
Lot No: **22**
Material: Brown-Yellow Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	14.0		
Cracking Crumbling Curling	Cracking & Curling		

Material Test Report

barnson.
Barnson Pty Ltd
Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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[redacted]y Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number: **37522-3**
Issue Number: 1
Date Issued: 08/06/2022
Client: Max Walker Constructions
P.O. Box 1151, Mudgee NSW 2850
Contact: Colleen Walker
Project Number: 37522
Project Name: Subdivision Developement
Project Location: 41D Robert Hoddle Grove, Bombira NSW
Work Request: 6270
Sample Number: D22-6270AC
Date Sampled: 23/05/2022
Dates Tested: 23/05/2022 - 07/06/2022
Sampling Method: AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection: Selected by Client
Sample Location: **Borehole 15 - Lot 23 Nelthorpe & Page Street Lat: -32.578518, Long: 149.606963, Depth: 800mm**
Lot No: **23**
Material: Brown Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	15.0		
Cracking Crumbling Curling	Cracking & Curling		

Material Test Report

barnson.
Barnson Pty Ltd
Dubbo Laboratory
16 L Yarrandale Road Dubbo NSW 2830
Phone: 1300 BARNSON
Email: jeremy@barnson.com.au

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[redacted]

Approved Signatory: Jeremy Wiatkowski
Geotechnical Technician
NATA Accredited Laboratory Number: 9605

Report Number: **37522-3**
Issue Number: 1
Date Issued: 08/06/2022
Client: Max Walker Constructions
P.O. Box 1151, Mudgee NSW 2850
Contact: Colleen Walker
Project Number: 37522
Project Name: Subdivision Developement
Project Location: 41D Robert Hoddle Grove, Bombira NSW
Work Request: 6270
Sample Number: D22-6270AD
Date Sampled: 23/05/2022
Dates Tested: 23/05/2022 - 07/06/2022
Sampling Method: AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection: Selected by Client
Sample Location: **Borehole 15 - Lot 23 Nelthorpe & Page Street Lat: -32.578518, Long: 149.606963, Depth: 2.0m**
Lot No: **23**
Material: Brown Sandy Silty CLAY

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	11.5		
Cracking Crumbling Curling	Cracking & Crumbling		

Appendix D – CSIRO Guide

Foundation Maintenance and Footing Performance:

A Homeowner's Guide

CSIRO
BTF 18
replaces
Information
Sheet 10/91

Buildings can and often do move. This movement can be up, down, lateral or rotational. The fundamental cause of movement in buildings can usually be related to one or more problems in the foundation soil. It is important for the homeowner to identify the soil type in order to ascertain the measures that should be put in place in order to ensure that problems in the foundation soil can be prevented, thus protecting against building movement.

This Building Technology File is designed to identify causes of soil-related building movement, and to suggest methods of prevention of resultant cracking in buildings.

Soil Types

The types of soils usually present under the topsoil in land zoned for residential buildings can be split into two approximate groups – granular and clay. Quite often, foundation soil is a mixture of both types. The general problems associated with soils having granular content are usually caused by erosion. Clay soils are subject to saturation and swell/shrink problems.

Classifications for a given area can generally be obtained by application to the local authority, but these are sometimes unreliable and if there is doubt, a geotechnical report should be commissioned. As most buildings suffering movement problems are founded on clay soils, there is an emphasis on classification of soils according to the amount of swell and shrinkage they experience with variations of water content. The table below is Table 2.1 from AS 2870, the Residential Slab and Footing Code.

Causes of Movement

Settlement due to construction

There are two types of settlement that occur as a result of construction:

- Immediate settlement occurs when a building is first placed on its foundation soil, as a result of compaction of the soil under the weight of the structure. The cohesive quality of clay soil mitigates against this, but granular (particularly sandy) soil is susceptible.
- Consolidation settlement is a feature of clay soil and may take place because of the expulsion of moisture from the soil or because of the soil's lack of resistance to local compressive or shear stresses. This will usually take place during the first few months after construction, but has been known to take many years in exceptional cases.

These problems are the province of the builder and should be taken into consideration as part of the preparation of the site for construction. Building Technology File 19 (BTF 19) deals with these problems.

Erosion

All soils are prone to erosion, but sandy soil is particularly susceptible to being washed away. Even clay with a sand component of say 10% or more can suffer from erosion.

Saturation

This is particularly a problem in clay soils. Saturation creates a bog-like suspension of the soil that causes it to lose virtually all of its bearing capacity. To a lesser degree, sand is affected by saturation because saturated sand may undergo a reduction in volume – particularly imported sand fill for bedding and blinding layers. However, this usually occurs as immediate settlement and should normally be the province of the builder.

Seasonal swelling and shrinkage of soil

All clays react to the presence of water by slowly absorbing it, making the soil increase in volume (see table below). The degree of increase varies considerably between different clays, as does the degree of decrease during the subsequent drying out caused by fair weather periods. Because of the low absorption and expulsion rate, this phenomenon will not usually be noticeable unless there are prolonged rainy or dry periods, usually of weeks or months, depending on the land and soil characteristics.

The swelling of soil creates an upward force on the footings of the building, and shrinkage creates subsidence that takes away the support needed by the footing to retain equilibrium.

Shear failure

This phenomenon occurs when the foundation soil does not have sufficient strength to support the weight of the footing. There are two major post-construction causes:

- Significant load increase.
- Reduction of lateral support of the soil under the footing due to erosion or excavation.
- In clay soil, shear failure can be caused by saturation of the soil adjacent to or under the footing.

GENERAL DEFINITIONS OF SITE CLASSES

Class	Foundation
A	Most sand and rock sites with little or no ground movement from moisture changes
S	Slightly reactive clay sites with only slight ground movement from moisture changes
M	Moderately reactive clay or silt sites, which can experience moderate ground movement from moisture changes
H	Highly reactive clay sites, which can experience high ground movement from moisture changes
E	Extremely reactive sites, which can experience extreme ground movement from moisture changes
A to P	Filled sites
P	Sites which include soft soils, such as soft clay or silt or loose sands; landslip; mine subsidence; collapsing soils; soils subject to erosion; reactive sites subject to abnormal moisture conditions or sites which cannot be classified otherwise

Tree root growth
Trees and shrubs that are allowed to grow in the vicinity of footings can cause foundation soil movement in two ways:

- Roots that grow under footings may increase in cross-sectional size, exerting upward pressure on footings.
- Roots in the vicinity of footings will absorb much of the moisture in the foundation soil, causing shrinkage or subsidence.

Unevenness of Movement

The types of ground movement described above usually occur unevenly throughout the building's foundation soil. Settlement due to construction tends to be uneven because of:

- Differing compaction of foundation soil prior to construction.
- Differing moisture content of foundation soil prior to construction.

Movement due to non-construction causes is usually more uneven still. Erosion can undermine a footing that traverses the flow or can create the conditions for shear failure by eroding soil adjacent to a footing that runs in the same direction as the flow.

Saturation of clay foundation soil may occur where subfloor walls create a dam that makes water pond. It can also occur wherever there is a source of water near footings in clay soil. This leads to a severe reduction in the strength of the soil which may create local shear failure.

Seasonal swelling and shrinkage of clay soil affects the perimeter of the building first, then gradually spreads to the interior. The swelling process will usually begin at the uphill extreme of the building, or on the weather side where the land is flat. Swelling gradually reaches the interior soil as absorption continues. Shrinkage usually begins where the sun's heat is greatest.

Effects of Uneven Soil Movement on Structures

Erosion and saturation
Erosion removes the support from under footings, tending to create subsidence of the part of the structure under which it occurs. Brickwork walls will resist the stress created by this removal of support by bridging the gap or cantilevering until the bricks or the mortar bedding fail. Older masonry has little resistance. Evidence of failure varies according to circumstances and symptoms may include:

- Step cracking in the mortar beds in the body of the wall or above/below openings such as doors or windows.
- Vertical cracking in the bricks (usually but not necessarily in line with the vertical beds or perpends).

Isolated piers affected by erosion or saturation of foundations will eventually lose contact with the bearers they support and may tilt or fall over. The floors that have lost this support will become bouncy, sometimes rattling ornaments etc.

Seasonal swelling/shrinkage in clay
Swelling foundation soil due to rainy periods first lifts the most exposed extremities of the footing system, then the remainder of the perimeter footings while gradually permeating inside the building footprint to lift internal footings. This swelling first tends to create a dish effect, because the external footings are pushed higher than the internal ones.

The first noticeable symptom may be that the floor appears slightly dished. This is often accompanied by some doors binding on the floor or the door head, together with some cracking of cornice mitres. In buildings with timber flooring supported by bearers and joists, the floor can be bouncy. Externally there may be visible dishing of the hip or ridge lines.

As the moisture absorption process completes its journey to the innermost areas of the building, the internal footings will rise. If the spread of moisture is roughly even, it may be that the symptoms will temporarily disappear, but it is more likely that swelling will be uneven, creating a difference rather than a disappearance in symptoms. In buildings with timber flooring supported by bearers and joists, the isolated piers will rise more easily than the strip footings or piers under walls, creating noticeable doming of flooring.

Trees can cause shrinkage and damage

As the weather pattern changes and the soil begins to dry out, the external footings will be first affected, beginning with the locations where the sun's effect is strongest. This has the effect of lowering the external footings. The doming is accentuated and cracking reduces or disappears where it occurred because of dishing, but other cracks open up. The roof lines may become convex.

Doming and dishing are also affected by weather in other ways. In areas where warm, wet summers and cooler dry winters prevail, water migration tends to be toward the interior and doming will be accentuated, whereas where summers are dry and winters are cold and wet, migration tends to be toward the exterior and the underlying propensity is toward dishing.

Movement caused by tree roots
In general, growing roots will exert an upward pressure on footings, whereas soil subject to drying because of tree or shrub roots will tend to remove support from under footings by inducing shrinkage.

Complications caused by the structure itself
Most forces that the soil causes to be exerted on structures are vertical – i.e. either up or down. However, because these forces are seldom spread evenly around the footings, and because the building resists uneven movement because of its rigidity, forces are exerted from one part of the building to another. The net result of all these forces is usually rotational. This resultant force often complicates the diagnosis because the visible symptoms do not simply reflect the original cause. A common symptom is binding of doors on the vertical member of the frame.

Effects on full masonry structures
Brickwork will resist cracking where it can. It will attempt to span areas that lose support because of subsided foundations or raised points. It is therefore usual to see cracking at weak points, such as openings for windows or doors.

In the event of construction settlement, cracking will usually remain unchanged after the process of settlement has ceased.

With local shear or erosion, cracking will usually continue to develop until the original cause has been remedied, or until the subsidence has completely neutralised the affected portion of footing and the structure has stabilised on other footings that remain effective.

In the case of swell/shrink effects, the brickwork will in some cases return to its original position after completion of a cycle, however it is more likely that the rotational effect will not be exactly reversed, and it is also usual that brickwork will settle in its new position and will resist the forces trying to return it to its original position. This means that in a case where swelling takes place after construction and cracking occurs, the cracking is likely to at least partly remain after the shrink segment of the cycle is complete. Thus, each time the cycle is repeated, the likelihood is that the cracking will become wider until the sections of brickwork become virtually independent.

With repeated cycles, once the cracking is established, if there is no other complication, it is normal for the incidence of cracking to stabilise, as the building has the articulation it needs to cope with the problem. This is by no means always the case, however, and monitoring of cracks in walls and floors should always be treated seriously.

Upheaval caused by growth of tree roots under footings is not a simple vertical shear stress. There is a tendency for the root to also exert lateral forces that attempt to separate sections of brickwork after initial cracking has occurred.

The normal structural arrangement is that the inner leaf of brickwork in the external walls and at least some of the internal walls (depending on the roof type) comprise the load-bearing structure on which any upper floors, ceilings and the roof are supported. In these cases, it is internally visible cracking that should be the main focus of attention, however there are a few examples of dwellings whose external leaf of masonry plays some supporting role, so this should be checked if there is any doubt. In any case, externally visible cracking is important as a guide to stresses on the structure generally, and it should also be remembered that the external walls must be capable of supporting themselves.

Effects on framed structures
Timber or steel framed buildings are less likely to exhibit cracking due to swell/shrink than masonry buildings because of their flexibility. Also, the doming/dishing effects tend to be lower because of the lighter weight of walls. The main risks to framed buildings are encountered because of the isolated pier footings used under walls. Where erosion or saturation cause a footing to fall away, this can double the span which a wall must bridge. This additional stress can create cracking in wall linings, particularly where there is a weak point in the structure caused by a door or window opening. It is, however, unlikely that framed structures will be so stressed as to suffer serious damage without first exhibiting some or all of the above symptoms for a considerable period. The same warning period should apply in the case of upheaval. It should be noted, however, that where framed buildings are supported by strip footings there is only one leaf of brickwork and therefore the externally visible walls are the supporting structure for the building. In this case, the subfloor masonry walls can be expected to behave as full brickwork walls.

Effects on brick veneer structures
Because the load-bearing structure of a brick veneer building is the frame that makes up the interior leaf of the external walls plus perhaps the internal walls, depending on the type of roof, the building can be expected to behave as a framed structure, except that the external masonry will behave in a similar way to the external leaf of a full masonry structure.

Water Service and Drainage

Where a water service pipe, a sewer or stormwater drainage pipe is in the vicinity of a building, a water leak can cause erosion, swelling or saturation of susceptible soil. Even a minuscule leak can be enough to saturate a clay foundation. A leaking tap near a building can have the same effect. In addition, trenches containing pipes can become watercourses even though backfilled, particularly where broken rubble is used as fill. Water that runs along these trenches can be responsible for serious erosion, interstrata seepage into subfloor areas and saturation.

Pipe leakage and trench water flows also encourage tree and shrub roots to the source of water, complicating and exacerbating the problem.
Poor roof plumbing can result in large volumes of rainwater being concentrated in a small area of soil:

- Incorrect falls in roof guttering may result in overflows, as may gutters blocked with leaves etc.
- Corroded guttering or downpipes can spill water to ground.
- Downpipes not positively connected to a proper stormwater collection system will direct a concentration of water to soil that is directly adjacent to footings, sometimes causing large-scale problems such as erosion, saturation and migration of water under the building.

Seriousness of Cracking

In general, most cracking found in masonry walls is a cosmetic nuisance only and can be kept in repair or even ignored. The table below is a reproduction of Table C1 of AS 2870.

AS 2870 also publishes figures relating to cracking in concrete floors, however because wall cracking will usually reach the critical point significantly earlier than cracking in slabs, this table is not reproduced here.

Prevention/ Cure

Plumbing
Where building movement is caused by water service, roof plumbing, sewer or stormwater failure, the remedy is to repair the problem. It is prudent, however, to consider also rerouting pipes away from the building where possible, and relocating taps to positions where any leakage will not direct water to the building vicinity. Even where gully traps are present, there is sometimes sufficient spill to create erosion or saturation, particularly in modern installations using smaller diameter PVC fixtures. Indeed, some gully traps are not situated directly under the taps that are installed to charge them, with the result that water from the tap may enter the backfilled trench that houses the sewer piping. If the trench has been poorly backfilled, the water will either pond or flow along the bottom of the trench. As these trenches usually run alongside the footings and can be at a similar depth, it is not hard to see how any water that is thus directed into a trench can easily affect the foundation's ability to support footings or even gain entry to the subfloor area.

Ground drainage
In all soils there is the capacity for water to travel on the surface and below it. Surface water flows can be established by inspection during and after heavy or prolonged rain. If necessary, a grated drain system connected to the stormwater collection system is usually an easy solution.

It is, however, sometimes necessary when attempting to prevent water migration that testing be carried out to establish watertable height and subsoil water flows. This subject is referred to in BTF 19 and may properly be regarded as an area for an expert consultant.

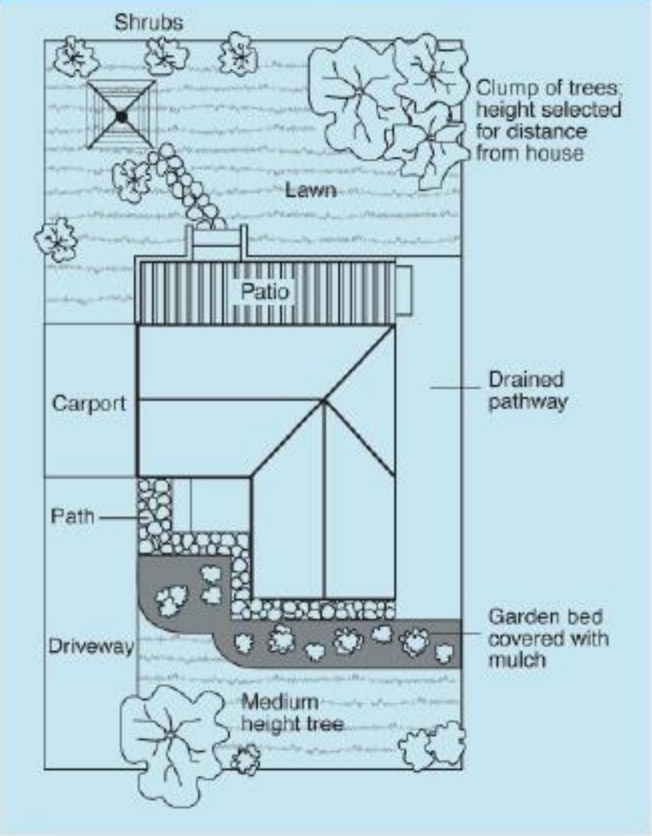
Protection of the building perimeter
It is essential to remember that the soil that affects footings extends well beyond the actual building line. Watering of garden plants, shrubs and trees causes some of the most serious water problems.

For this reason, particularly where problems exist or are likely to occur, it is recommended that an apron of paving be installed around as much of the building perimeter as necessary. This paving

CLASSIFICATION OF DAMAGE WITH REFERENCE TO WALLS

Description of typical damage and required repair	Approximate crack width limit (see Note 3)	Damage category
Hairline cracks	<0.1 mm	0
Fine cracks which do not need repair	<1 mm	1
Cracks noticeable but easily filled. Doors and windows stick slightly	<5 mm	2
Cracks can be repaired and possibly a small amount of wall will need to be replaced. Doors and windows stick. Service pipes can fracture. Weathertightness often impaired	5–15 mm (or a number of cracks 3 mm or more in one group)	3
Extensive repair work involving breaking-out and replacing sections of walls, especially over doors and windows. Window and door frames distort. Walls lean or bulge noticeably, some loss of bearing in beams. Service pipes disrupted	15–25 mm but also depend on number of cracks	4

Gardens for a reactive site



should extend outwards a minimum of 900 mm (more in highly reactive soil) and should have a minimum fall away from the building of 1:60. The finished paving should be no less than 100 mm below brick vent bases.

It is prudent to relocate drainage pipes away from this paving, if possible, to avoid complications from future leakage. If this is not practical, earthenware pipes should be replaced by PVC and backfilling should be of the same soil type as the surrounding soil and compacted to the same density.

Except in areas where freezing of water is an issue, it is wise to remove taps in the building area and relocate them well away from the building – preferably not uphill from it (see BTF 19).

It may be desirable to install a grated drain at the outside edge of the paving on the uphill side of the building. If subsoil drainage is needed this can be installed under the surface drain.

Condensation

In buildings with a subfloor void such as where bearers and joists support flooring, insufficient ventilation creates ideal conditions for condensation, particularly where there is little clearance between the floor and the ground. Condensation adds to the moisture already present in the subfloor and significantly slows the process of drying out. Installation of an adequate subfloor ventilation system, either natural or mechanical, is desirable.

Warning: Although this Building Technology File deals with cracking in buildings, it should be said that subfloor moisture can result in the development of other problems, notably:

- Water that is transmitted into masonry, metal or timber building elements causes damage and/or decay to those elements.
- High subfloor humidity and moisture content create an ideal environment for various pests, including termites and spiders.
- Where high moisture levels are transmitted to the flooring and walls, an increase in the dust mite count can ensue within the living areas. Dust mites, as well as dampness in general, can be a health hazard to inhabitants, particularly those who are abnormally susceptible to respiratory ailments.

The garden

The ideal vegetation layout is to have lawn or plants that require only light watering immediately adjacent to the drainage or paving edge, then more demanding plants, shrubs and trees spread out in that order.

Overwatering due to misuse of automatic watering systems is a common cause of saturation and water migration under footings. If it is necessary to use these systems, it is important to remove garden beds to a completely safe distance from buildings.

Existing trees

Where a tree is causing a problem of soil drying or there is the existence or threat of upheaval of footings, if the offending roots are subsidiary and their removal will not significantly damage the tree, they should be severed and a concrete or metal barrier placed vertically in the soil to prevent future root growth in the direction of the building. If it is not possible to remove the relevant roots without damage to the tree, an application to remove the tree should be made to the local authority. A prudent plan is to transplant likely offenders before they become a problem.

Information on trees, plants and shrubs

State departments overseeing agriculture can give information regarding root patterns, volume of water needed and safe distance from buildings of most species. Botanic gardens are also sources of information. For information on plant roots and drains, see Building Technology File 17.

Excavation

Excavation around footings must be properly engineered. Soil supporting footings can only be safely excavated at an angle that allows the soil under the footing to remain stable. This angle is called the angle of repose (or friction) and varies significantly between soil types and conditions. Removal of soil within the angle of repose will cause subsidence.

Remediation

Where erosion has occurred that has washed away soil adjacent to footings, soil of the same classification should be introduced and compacted to the same density. Where footings have been undermined, augmentation or other specialist work may be required. Remediation of footings and foundations is generally the realm of a specialist consultant.

Where isolated footings rise and fall because of swell/shrink effect, the homeowner may be tempted to alleviate floor bounce by filling the gap that has appeared between the bearer and the pier with blocking. The danger here is that when the next swell segment of the cycle occurs, the extra blocking will push the floor up into an accentuated dome and may also cause local shear failure in the soil. If it is necessary to use blocking, it should be by a pair of fine wedges and monitoring should be carried out fortnightly.

This BTF was prepared by John Lewer FAIB, MIAMA, Partner, Construction Diagnosis.

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CSIRO PUBLISHING PO Box 1139, Collingwood 3066, Australia
Freecall 1800 645 051 Tel (03) 9662 7666 Fax (03) 9662 7555 www.publish.csiro.au
Email: publishing.sales@csiro.au