



barnson.
DESIGN . PLAN . MANAGE

Site and Soil Assessment for On-site Effluent Management System

Client: Jorge Diaz
Site Address: 1808 Windeyer Road
Windeyer, NSW 2850

26 March 2025

Our Reference : 46937-ER01_A

© Barnson Pty Ltd 2025. Confidential.

List of Contents

1.0 System Overview	5
2.0 Introduction.....	6
2.1 Overview.....	6
2.2 Key References	6
2.3 Onsite Effluent Management System	6
2.0 Site and Soil Evaluation	10
2.1 Site Evaluators Details	10
2.2 Site Information.....	10
2.3 Desktop Assessment.....	11
2.4 Groundwater Review.....	12
2.5 Surface Water Review	12
2.6 Field Assessment Information	15
2.7 Soil Assessment	16
3.0 Site and Soil Limitation Assessment.....	17
4.0 System Requirements.....	19
4.1 Mid-Western Regional Council Setback Requirements.....	19
All Land Application Systems.....	19
Absorption Systems.....	19
4.2 Design Allowances – AS/NZS1547:2012 Table H1.....	19
5.0 Septic Tank Selection and Calculation.....	20
5.1 Silver Book/ NSW Health Guidelines	20
5.2 AS/NZS 1547:2012 Requirements	20
5.3 System Recommendations.....	21
6.0 Effluent Management	22
6.1 Hydraulic Loading Calculation	22
Proposed Development.....	22
6.2 Design Recommendations.....	23
7.0 Recommendations.....	24

List of Tables

Table 1 : System Overview	5
Table 2: Details	10
Table 3: Site Particulars	10
Table 4: Desktop Assessment Details	11
Table 5: Groundwater Review	12
Table 6: Site Assessment Details	15
Table 7: Soil Assessment Details	16
Table 8: Site Limitation Assessment	17
Table 9: Soil Limitation Assessment	18
Table 10: System Selection Details	21

List of Figures

Figure 1 – Site Location Plan	7
Figure 2 – Buffer and Setback Plan	8
Figure 3 – Buffer and Setback Plan	9
Figure 4 – Groundwater Bore Locations	13
Figure 5 – Groundwater Vulnerability Map GRV_006	14

Appendices

APPENDIX A	Water Balance	26
APPENDIX B	Borehole Logs & Laboratory Results	28
APPENDIX C	Site Setback Requirements	31
APPENDIX D	Absorption Bed Concept Plans	36
APPENDIX E	List of Plates	40

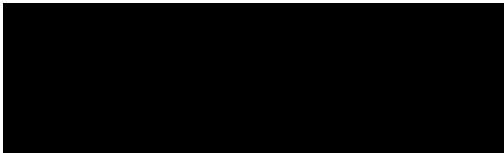
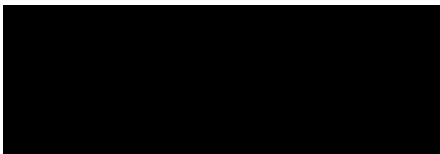
DISCLAIMER

This report has been prepared solely for Jorge Diaz in accordance with the scope provided by the client and for the purpose(s) as outlined throughout this report.

Installation must be by a licensed plumber and Barnson will not be liable for the incorrect installation and/or construction of the system. Installation and construction of the system must hold true to the design recommendations presented in this report. Installation should be in accordance with the prescriptions within AS 1547:2012.

Unless otherwise stated in this report, Barnson has not verified the accuracy or completeness of the data retrieved from online databases and guidance documents. The recommendations for the proposed system as presented in this report are based on historical data obtained for the area. Barnson will not be liable in relation to incorrect recommendations should any information provided by the client be incorrect or have been concealed, withheld, misrepresented or otherwise not fully disclosed.

The accuracy of the advice provided in this report may be limited by unobserved variations in ground conditions across the site in areas between and beyond test locations and by any restrictions in the sampling and testing which was able to be carried out, as well as by the amount of data that could be collected given the project and site constraints. These factors may lead to the possibility that actual ground conditions and materials behaviour observed at the test locations may differ from those which may be encountered elsewhere on the site. If the sub-surface conditions are found to differ from those described in this report, we should be informed immediately to evaluate whether recommendations should be reviewed and amended if necessary.

Project:	Lot 231 DP1142826, 1808 Windeyer Road, Windeyer NSW 2850	
Client:	Jorge Diaz	
Project Number:	46937	
Report Reference:	46937-ER01_A	
Date:	21/03/2025	
Prepared by:		Reviewed by:
		
Jeremy Wiatkowski AdvDip Laboratory Operations Senior Laboratory Technician		Andrew Ruming BSc Environmental Geologist

1.0 SYSTEM OVERVIEW

The following table provides a summary of the information for a sustainable onsite effluent management system proposed at Lot 231 DP1142826, 1808 Windeyer Road, Windeyer NSW 2850. The sections of this report that follow, provide site specific details justifying the recommended system.

Table 1 : System Overview

Site Assessor	Jeremy Wiatkowski
Client	Jorge Diaz
Site Location	"Lot 231 DP1142826", 1808 Windeyer Road, Windeyer NSW
Max People Usage	Maximum Usage of 2 people
Proposed Amenities	Kitchen & Bathroom
Water Source	Rainwater roof collection
Estimated Daily Flow (L/day)	240L/Day based on 2 people at 120L/person/day
Tank Recommendation	Standard Septic Tank
Tank Capacity	As per section 6.3 the minimum size tank required is 3000L
Sub Soil Assessment Class	Field assessment and subsequent laboratory tests have classed the subsoil as category 4, as shown in section 3.7.
Sub Soil Recommended Hydraulic Loading mm/day (DIR/DLR)	Bed/trench systems in category 4 soils have a design-loading rate of 10mm/day. (Refer to Table 7).
Recommended Effluent Application Type	Due to the category 4 soil (Clay Loams) it is recommended that an absorption bed be utilised to disperse effluent.
Effluent Design Criteria	<i>As per section 7.0 the minimum application area was determined by calculating the requirements of hydraulic loading. As shown 1 absorption bed, of 12m long x 2m wide is required to dispose of the proposed hydraulic load.</i>
Additional Notes	During construction gypsum to be applied at 1 kg/m ² to the base of the excavated bed/trench to prevent the soil dispersing. The bed/trench shall be closed in, as soon as possible to protect the gypsum from raindrop impact.

2.0 INTRODUCTION

2.1 Overview

Barnson Pty Ltd on behalf of Jorge Diaz has prepared this report for submission to Mid-Western Regional Council. This report provides direction for sustainable on-site effluent management for a proposed development, on Lot 231 DP1142826, at 1808 Windeyer Road, Windeyer NSW (refer **Figure 1 & 2**).

2.2 Key References

The following key references were utilised as part of this assessment:

- AS/NZS 1547:2012. *On-site Domestic Wastewater Management*;
- NSW Government 1998. *On site Sewerage Management for Single Households* (The Silver Book/OSMSH);
- NSW Government 2000. *The Easy Septic Tank Guide*. Developed by Social Change Media for the NSW Department of Local Government;
- NSW Health, 2016. 'Septic Tank and Collection Well Accreditation Guidelines';
- Mid-Western Regional Council Local Environment Plan, 2012;
- Mid-Western Regional Council 'On-Site Sewage Management Plan' (2008);
- Murphy B.W. & Lawrie J.W. 1998. Soil Landscapes of the Dubbo 1:250 000 Sheet Report, DLWC.
- Sydney Catchment Management Authority, 2023. *Designing and Installing On-Site Wastewater Systems*;

2.3 Onsite Effluent Management System

The onsite effluent management system proposed for this site consists of a standard septic tank with primary treated effluent disposed into absorption beds. **Figure 1 & 2** illustrates the site location. **Figure 3** illustrates the proposed buffer, setback areas and proposed application area.

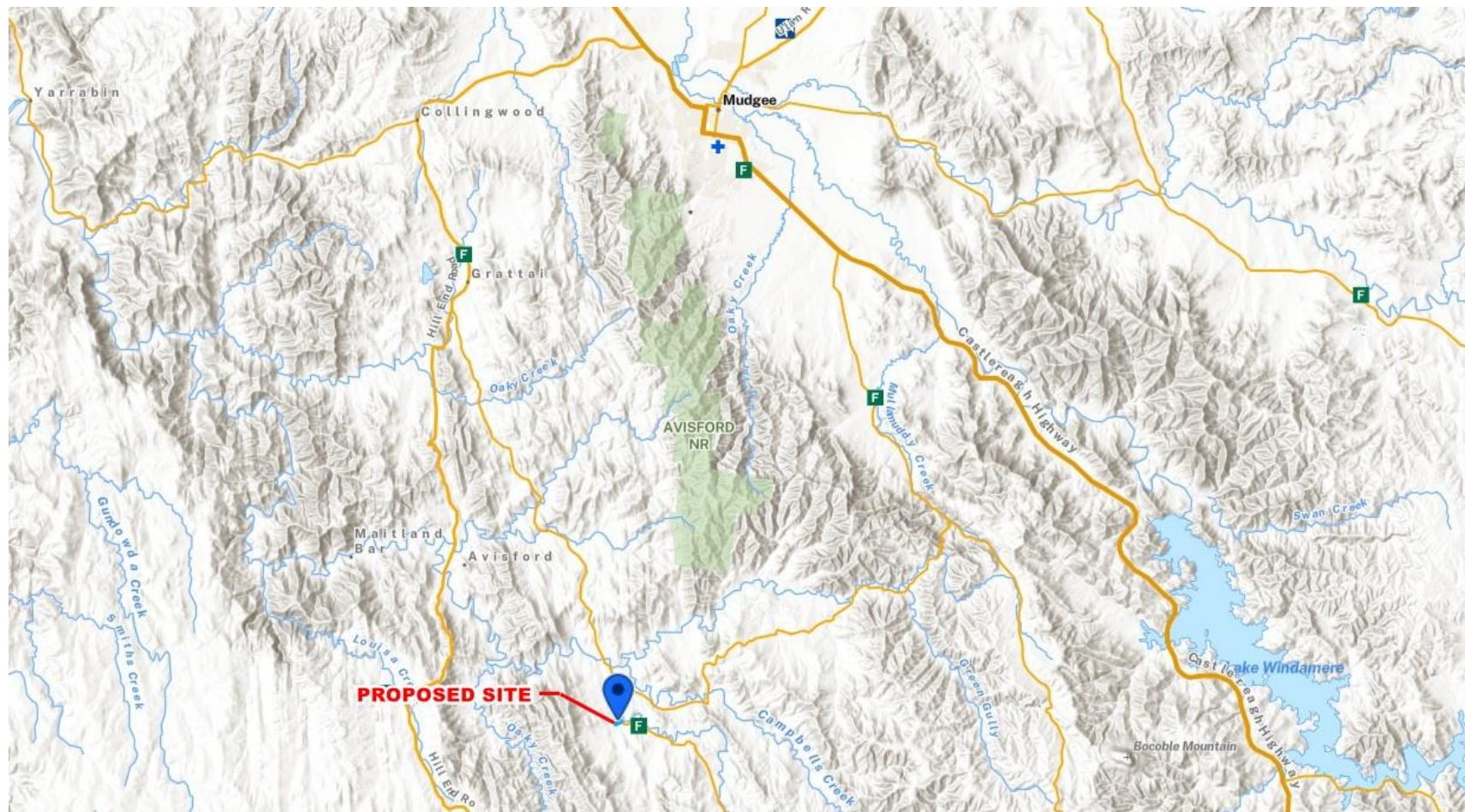


Figure 1 – Site Location Plan

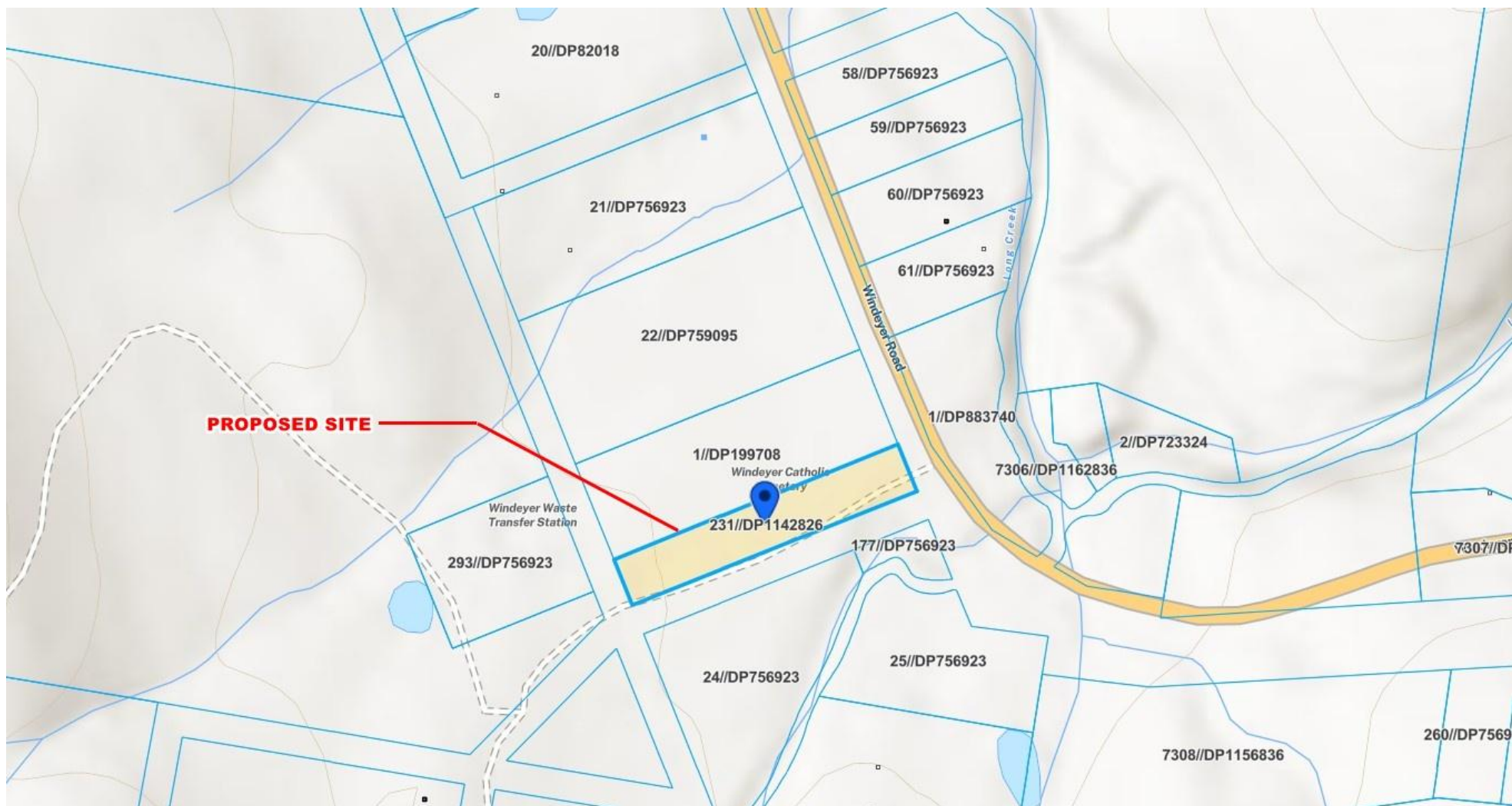


Figure 2 – Buffer and Setback Plan



Figure 3 – Buffer and Setback Plan

BARNSON PTY LTD

phone 1300 BARNSON (1300 227 676)
email generalenquiry@barnson.com.au
web barnson.com.au

THIS DRAWING IS TO BE READ IN CONJUNCTION WITH GENERAL BUILDING DRAWINGS, SPECIFICATIONS & OTHER CONSULTANT DRAWINGS APPLICABLE TO THIS PROJECT. ALL DIMENSIONS IN MILLIMETRES, DO NOT SCALE. DIMENSIONS TO BE CHECKED ON SITE BEFORE COMMENCEMENT OF WORK. REPORT DISCREPANCIES TO BARNSON PTY LTD. NO PART OF THIS DRAWING MAY BE REPRODUCED IN ANY WAY WITHOUT THE WRITTEN PERMISSION OF BARNSON PTY LTD.

Project

ONSITE SEWAGE DISPOSAL

Client
JORGE DIAZ

Drawing Title
PROPOSED SITE BUFFERS

Site Address

**1808 WINDEYER ROAD
WINDEYER NSW**

Certification

Design JW
Drawn JW
Check AR

Original Size A4
Revision A

Project No
Drawing No

46937
GD01

2.0 SITE AND SOIL EVALUATION

2.1 Site Evaluators Details

The following table provides an overview of the evaluator's particulars.

Table 2: Details

Name / Role	Jeremy Wiatkowski
Role/ Qualifications	Geotechnical Technician
Company	Barnson Pty Ltd
Company Address	1/36 Darling Street Dubbo NSW 2830
Contact Details	1300 BARNSON
Date of Assessment	07/03/2025

2.2 Site Information

The following table provides an overview of the site information.

Table 3: Site Particulars

Address/Locality	1808 Windeyer Road, Windeyer NSW Lot 231 DP1142826
Local Government Area	Mid-Western Regional Council
Owner	Jorge Diaz
Block Configuration	Approximately 0.7 ha
Intended Water Supply	Rainwater roof collection supplied
Intended Power Supply	Supplied
Local Experience	Care needs to be taken to minimise runoff and erosion. Systems commonly malfunction due to lack of ongoing maintenance. The system is to be inspected and maintained regularly in accordance with manufacturer details, Council requirements, and prescriptions identified in this report.

2.3 Desktop Assessment

The following information was obtained via desktop review of the site.

Table 4: Desktop Assessment Details

Climate Overview¹	Annual Average Rainfall for Mudgee is 678.8mm. Warm summers with large evaporative deficit, cool winters with small evaporative deficit. The mean summer monthly rainfall (January) is 68.2mm. The mean winter rainfall (July) is 53mm.	
Soil Landscape Reference²	Area has been mapped within the 'Mookerawa" Landscape Group. Yellow Soloths are dominant in the area and Red Podzolic Soils are common in the area.	
	Surface Conditions	Hardsetting, copious quartz float on lower slopes
	Drainage	Imperfectly drained
	Available water holding capability	Moderate to low
	Water table depth	May be perched
	Depth to bedrock	>150-200 cm
	Flood hazard	Low to moderate (drainage flats)
	Expected Nutrient deficiencies	Nitrogen, Phosphorous
	Soil Salinity	Low to moderate
	Erosion Hazard	Moderate to high
Underlying Geology³	<i>"Thinly to thickly bedded, muddy, crystal-lithic, rhyolitic to rhyodacitic volcanoclastic sandstone interbedded with lesser tuff, siltstone, phyllitic shale and paraconglomerate".</i>	
Groundwater Review	One water bore was found within 500m of the proposed site, as illustrated in Figure 4 . The area is mapped as being groundwater vulnerable as per the Mid-Western Regional Council LEP map GRV_006 Figure 5 .	

¹ Bureau of Meteorology online Climate Data website

² NSW Soil and Land Information System

³ Dubbo 1:250000

2.4 Groundwater Review

One water bore was identified as occurring within the general area of the allotment. Information relating to historic groundwater report details on water bearing zones and standing water levels is provided in the table below.

Table 5: Groundwater Review

Groundwater Bore Reference	Total Depth (m)	Water Bearing Zones (m)	Standing Water Level (m)	Yield (L/s)	Salinity Description
GW801538 Bore Domestic Approximately 400m North of Site	36.00	16.00-16.20	9.00	0.25	Not Provided
		22.00-22.10	9.00	0.62	

Using available groundwater information from local bores, it can be determined that in the local vicinity the standing water level is greater than 9m below the ground surface and the water bearing zones are greater than 16m below the ground surface.

No groundwater was encounter during the site investigation. From this information it can be determined that in this locality, subsequent contamination by primary treated effluent is not a risk factor.

2.5 Surface Water Review

The proposed site drains towrds the south-east. Long Creek is approximately 100m east of the existing building. Wingraves Gully is approximately 50m south of the existing building.

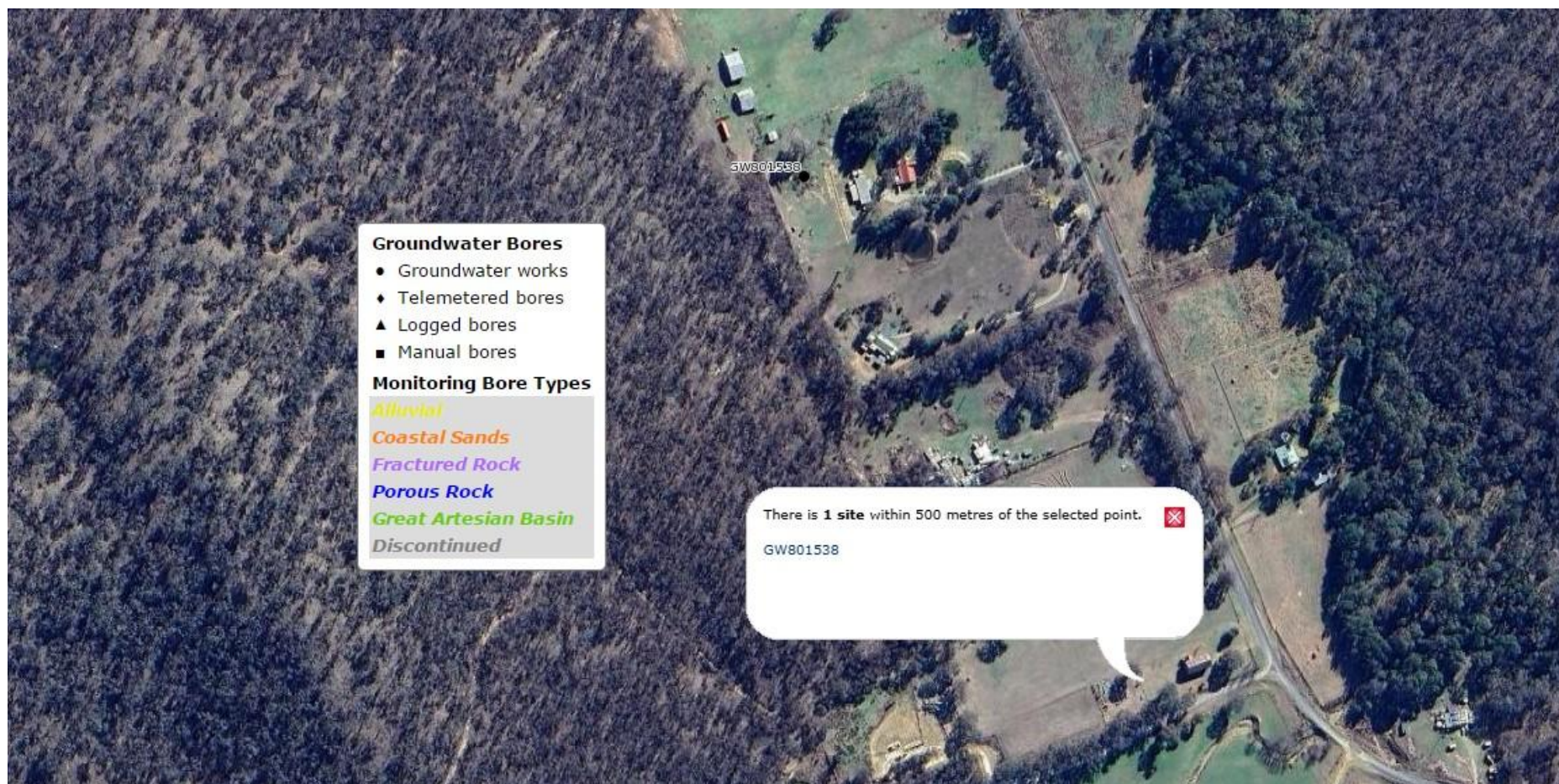


Figure 4 – Groundwater Bore Locations

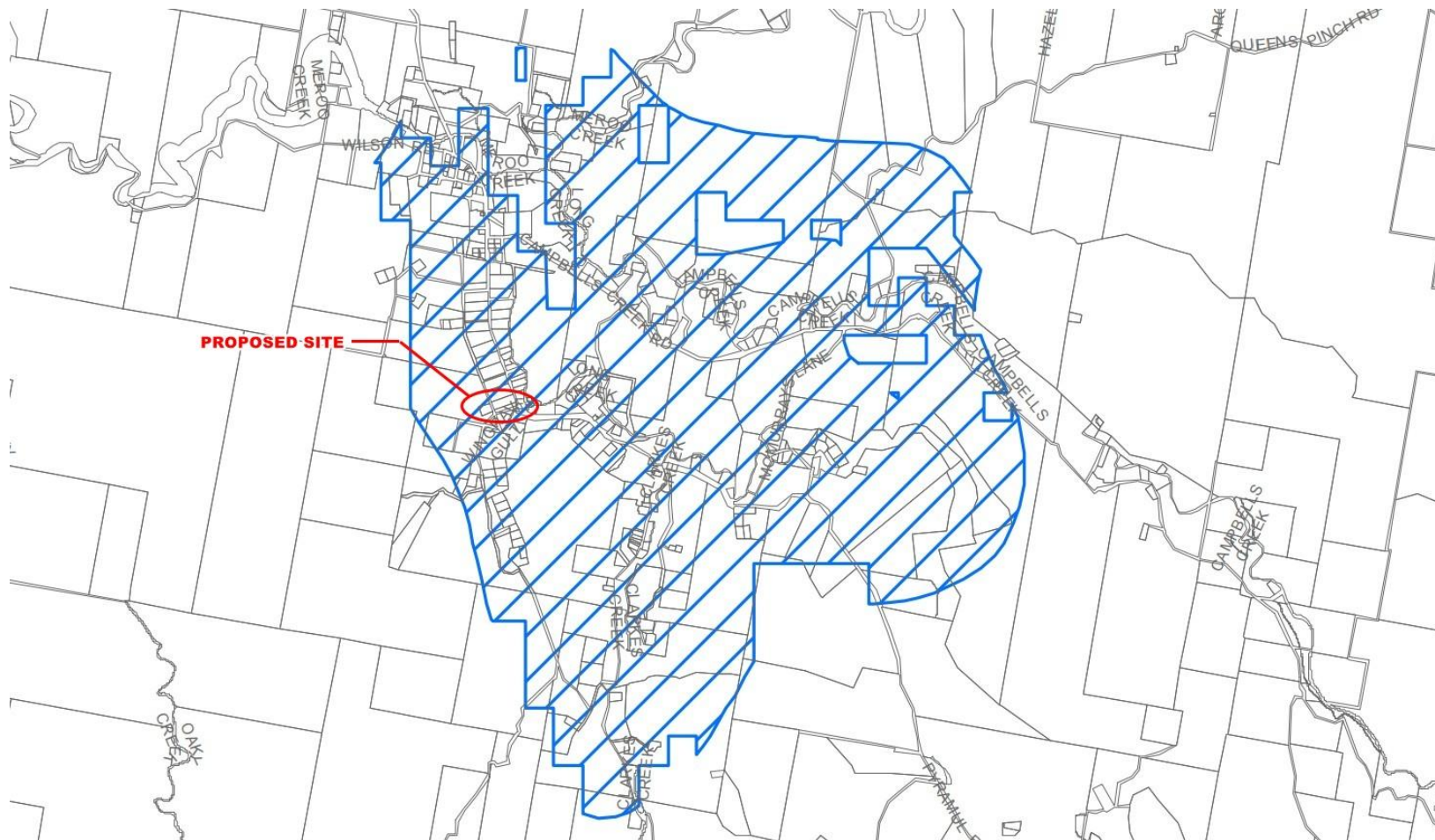


Figure 5 – Groundwater Vulnerability Map GRV_006

2.6 Field Assessment Information

A field inspection was conducted on 07/03/2025. The following table provides detail on the site assessment as well as the field and laboratory results.

Table 6: Site Assessment Details

Water Balance Attached		See <i>Appendix A</i>
Exposure		Good exposure.
Slope		The site has a slight slope to the south-east
Run-On		None
Seepage		None
Erosion Potential		Low due to vegetation cover.
Site Drainage		The proposed site drains towards the south-east. Long Creek is approximately 100m east of the existing building. Wingraves Gully is approximately 50m south of the existing building
Fill		None encountered
Surface rock/Outcrops		None encountered
Is there sufficient land area for:	Application system, including buffers	Yes
	Reserve application system	Yes

2.7 Soil Assessment

A soil sample was collected and returned to Barnson Pty Ltd for analysis on 07/03/2025. The sample was collected at a depth of 800mm during the site inspection as per AS1289.1.2.1.6.5.3. Laboratory report with results are provided at Appendix B. Field assessment parameters were also obtained. The following table provides detail on both field and laboratory assessment results.

Table 7: Soil Assessment Details

Depth to bedrock or hardpan via field assessment		>1.5m
Depth to high soil water table via field assessment		>1.5m
Soil Analysis	pH – subsoil CaCl ₂ (lab), subsoil	4.9
	Electrical conductivity (dS/m) - E _{Ce}	1.8
	Emerson Test Result –subsoils (Lab)	6
	Liquid Limit, Plastic Limit, Plasticity Index, Linear Shrinkage. (%)	LL = 26 PL = 14 PI = 12 – Low Plasticity LS = 3.5 – Low Reactivity See Borelog in Appendix B
	Estimated Soil Category–topsoil, subsoil A, subsoil B,	3,3,4
	Structure massive, weak, high, moderate, strong (Field)	High/Moderate Structured
	Soil Profile description	See Borelog in Appendix B
	Sub soil Permeability (from table 5.2 of AS 1547:2012)	0.5-1.5(k _{sat}) (m/d) 20.8-62.5 (mm/hr) (Infiltration is Moderate)
	Recommended Hydraulic Loading for disposal system (from Table 5.2 of AS 1547:2012)	10mm per day (For effluent disposal beds/trenches)

3.0 SITE AND SOIL LIMITATION ASSESSMENT

The following two limitation tables are a standardised guide to the site and soil characteristics which may limit the suitability of the site for effluent disposal and which require attention through specific management practises. The tables have been reproduced from the NSW Government endorsed 'On-Site Sewerage Management for Single Households' (1998), **Tables 8 and 9**. The highlighted categories represent site and soil conditions of the land covered in this report.

Table 8: Site Limitation Assessment

Site Feature	Relevant System	Minor Limitation	Moderate Limitation	Major Limitation	Restrictive Feature
Flood Potential	All land application systems	> 1 in 20 years		Frequent below 1 in 20 years	Transport in wastewater off site
	All treatment application systems	Components above 1 in 100 years		Components below 1 in 100 years	Transport in wastewater off site system failure
Exposure	All land application systems	High sun and wind exposure		Low sun and wind exposure	Poor evaporation transpiration
Slope %	Surface Irrigation	0-6	6-12	>12	Runoff, erosion potential
	Sub-surface irrigation	0-10	10-20	>20	Runoff, erosion potential
	Absorption	0-10	10-20	>20	Runoff, erosion potential
Landform	All systems	Hillcrests, convex side slopes and plains	Concave side slopes and foot slopes	Drainage plains and incised channels	Groundwater pollution hazard, resurfacing hazard
Run-on and upslope seepage	All land Application Areas	None-low	Moderate	High, diversion not practical	Transport of wastewater off site
Erosion potential	All land application systems	No sign of erosion potential		Indications of erosion e.g. rills, mass failure	Soil degradation and off-site impact
Site drainage	All land application systems	No visible signs of surface dampness		Visible signs of surface dampness, such as moisture-tolerant veg	Groundwater pollution hazard, resurfacing hazard
Fill	All systems	No fill	Fill present		Subsidence
Land area	All systems	Area available		Area not available	Health and pollution risk
Rock and rock outcrop	All land application systems	<10%	10-20%	>20%	Limits system performance
Geology	All land application systems	None		Major geological discontinuities, fractured or highly porous regolith	Groundwater pollution hazard

Table 9: Soil Limitation Assessment

Soil feature	Relevant system	Minor limitation	Moderate limitation	Major limitation	Restrictive feature
Depth to bedrock or hardpan (m)	Surface and sub-surface irrigation	> 1.0	0.5-1.0	< 0.5	Restricts plant growth
	Absorption	> 1.5	1.0-1.5	< 1.0	Groundwater pollution hazard
Depth to seasonal water table (m)	Surface and sub-surface irrigation	> 1.0	0.5-1.0	< 0.5	Groundwater pollution hazard
	Absorption	> 1.5	1.0-1.5	< 1.0	Groundwater pollution hazard
Permeability Category	Surface and sub-surface irrigation	2b, 3 and 4	2a, 5	1 and 6	Excessive runoff and waterlogging
	Absorption	3, 4		1, 2, 5 and 6	Percolation
Coarse fragments %	All systems	0-20	20-45	>40	Restricts plant growth, affects trench installation
Bulk density (g/cc) SL L, CL C	All land application systems	< 1.8 < 1.6 < 1.4		> 1.8 > 1.6 >1.4	restricts plant growth, indicator of permeability
pH	All land application systems	> 6.0	4.5-6.0	-	Reduces plant growth
Electrical conductivity (dS/m)	All land application systems	<4	4-8	>8	Restricts plant growth
Sodicity (ESP)	Irrigation 0-40cm; absorption 0-1.2mtr	0-5	5-10	> 10	Potential for structural degradation
CEC mequiv/100g	Irrigation systems	> 15	5-15	< 5	Nutrient leaching
P sorption kg/ha	All land application systems	> 6000	2000-6000	< 2000	Capacity to immobilise P
Modified Emerson Aggregate Test – (dispersiveness)	All land application systems	Class 3, 4	Class 2	Class 1	Potential for Structural degradation.

4.0 SYSTEM REQUIREMENTS

4.1 Mid-Western Regional Council Setback Requirements

The Mid-Western Regional Council 'On-Site Sewage Management Plan' (2008), provides recommended buffer distances. For this design, the following must be taken into consideration.

All Land Application Systems

- 80m to permanent surface waters (e.g. river, streams, lakes, etc.);
- 50m to domestic groundwater well on applicant's property and 200m to any groundwater well located on a neighbouring property;
- 40m to other waters (e.g. farm dams, intermittent waterways and drainage channels, etc.)

Absorption Systems

- 12m if area up-grade and 6m if area down gradient of property boundary;
- 6m if area is up-gradient and 3m if area is down gradient of swimming pools, driveways and building.

Other site setback requirement as per AS/NZS 1547:2012 are provided in **Appendix C**.

Actual siting of the effluent application area is the responsibility of a licenced plumber. The prescribed buffer areas/setbacks are to be adhered to unless otherwise specified by Council.

4.2 Design Allowances – AS/NZS1547:2012 Table H1

In accordance with AS/NZS1547:2012 Table H1, the recommended design flow allowance for use in Australia, using on site rainwater roof collection supply is 120L/person/day. The maximum number of persons is 2 people.

5.0 SEPTIC TANK SELECTION AND CALCULATION

5.1 Silver Book/ NSW Health Guidelines

The '[On-Site Sewerage Management for Single Households' \(1998\)](#) guideline is based on the NSW Health guideline for septic tank capacity. Therefore, the calculation is the same.

Primary effluent treatment will be provided by a NSW Health accredited septic tank. The [NSW Health 'Septic Tank and Collection Well Accreditation Guidelines' \(2016\)](#), set a sludge allowance of 1550L irrespective of the number of persons or which the septic tank is to be designed. It should be noted that in accordance with this guideline, a septic tank designed for a minimum of 5 persons needs to be de-sludge approximately every 4 years.

The general formula to calculate the minimum septic tank capacity in litres is:

$$S + (DF \times N) = C$$

$$\text{Sludge} + (\text{Daily Flow} \times \text{No. of Persons}) = \text{Capacity of the tank}$$

When DF = 120L/per person/per day and N =2, therefore DF x N =**240L**

$$1550L + 240L = 1790L$$

Table 2 in the NSW Health Guidelines provides a minimum of 2300L tank capacity.

5.2 AS/NZS 1547:2012 Requirements

A more conservative approach is outlined in AS/NZS1547:2012, Appendix J. A more conservative figure of 200L per person for all waste tanks is provided, giving a daily flow volume of 400L for the proposed development. Therefore, a minimum capacity tank of **3000L** is required for a development with a design flow of up to 1000L. This conservative rate is to ensure that the unit has capacity to cope with peak discharge rates or for temporary or unusual overloads and includes no allowance for food waste disposal units. This tank design capacity also allows for the storage of sludge and scum at a rate of 80L/person/year. It should be noted that the higher cost of installing a larger septic tank may be offset by a reduced pump out frequency. Too frequent pump out removes microorganisms needed for degradation of wastewater solids. The longer pump out interval has beneficial implications for conservation of resources in that the volume of seepage requiring treatment and disposal can be reduced significantly.

5.3 System Recommendations

The following table provides details on the system selection.

Table 10: System Selection Details

Consideration of connection to centralised sewerage system	Distance to sewer	>10km
	Potential for future connection?	None planned
	Potential for reticulated water?	None planned
Expected Wastewater volume (litres/day)	Proposed Development –proposed maximum occupancy of 2 people. Typical wastewater design flow is 120L/person per day in accordance with Table H3 of AS/NZS1547:2012 for households with full water reduction facilities, supplied by rainwater roof collection supply. Therefore, 2 people at 120L per person per day gives a total load of 240L/day	
Type of Treatment system best suited	3000L septic tank system– as per NSW Health accredited system - http://www.health.nsw.gov.au/environment/domesticwastewater/Pages/stcw.aspx with primary treated effluent to be distributed to an Absorption Bed	

Water conservation measures should be adapted to the greatest extent possible in the proposed development, particularly in relation to the high water-use activities of showering, clothes washing and toilet flushing. AAA rated plumbing appliances and fittings should be used. Measures including use of front-loading washing machines, low volume shower roses and dual flush toilets can reduce water usage by 30-40%. Detergents low in phosphorous and sodium should be used as much as possible. Following these measures will ensure the greatest lifespan for this effluent treatment and disposal system.

6.0 EFFLUENT MANAGEMENT

Barnson Pty Ltd has analysed the proposed on-site waste management system in accordance with the NSW Government endorsed 'Silver Book' (1998) and the ANZ Standard 1547:2012 On-site Domestic Wastewater Management', with additional advice sought from the Sydney Catchment Management Authority 'Designing and installing On-site Wastewater Systems' 2023 guideline. For this site, given the climate and soil constraints, absorption is considered the most appropriate effluent management device.

6.1 Hydraulic Loading Calculation

Given the proposed development will be connected by rainwater roof collection supply, the daily flow (Q) for the system is calculated as 240L/per day.

The required bed area shall be determined from the following relationship:

$$\text{Length of Absorption Bed} = (Q) / (DLR \times W)$$

Proposed Development

Where Q = 240L, DLR =10 mm/day (Table L1 AS 1547:2012 –Conservative Rate),

W (Width) = 2m

$$\begin{aligned} \text{Length of Bed} &= \left(\frac{240}{10 \times 2m} \right) \\ &= 12m \end{aligned}$$

Therefore, from the above calculation, 1 x 12m long, 2m wide bed will be required for the proposed development with a maximum ususage of 2 people.

6.2 Design Recommendations

Common failures of beds/trenches are often caused by poor installation practices. In addition to specifications outlined in AS/NZS 1547:2012, the following points should also be considered in the bed/trench design/construction which to meet the *minimum* dimensions of **1 bed, 12m long and 2m wide**.

- Beds/trenches are to be built along the contour to ensure even distribution and avoid any section being over loaded;
- Avoid cutting beds into weakened ground;
- Construction is to take place during fine weather. If it rains beds are to be completely covered to protect them from rain damage;
- Where the beds/trenches are dug by an excavator in clay soils, the bed walls are to be scarified to remove any smearing caused by the excavator bucket;
- During construction gypsum to be applied at 1 kg/m² to the base of the trench or bed to prevent the soil dispersing. The trench shall be closed in, as soon as possible to protect the gypsum from raindrop impact.
- All distribution pipes and arches should be laid in accordance with the manufactures instructions;
- If two beds or more are utilised, ensure effluent is distributed evenly via a splitter box or sequencing valve or other appropriate method;
- All distribution pipes and arches should be laid in accordance with the manufactures instructions;
- Consideration can be given to using a pressure dosed system, which would allow for a better, more even distribution of effluent along the trench, and prolong trench life;
- Inspection ports shall be provided for the beds/trenches system. The inspection port shall be installed so as to facilitate monitoring of the effluent level in each trench;
- Trenches/Beds may be gravity fed or pressure dosed using pumps or dosing siphons;
- Vegetation cover must be well maintained to ensure strong growth for maximum uptake of transpiration. The surrounding landscape and vegetation must also be maintained to minimise shading and maximise exposure.
- The beds/trenches should be in an enclosed area protected from vehicle movement or livestock that can cause compaction and premature trench failure;
- The beds/trenches are to be constructed along the contour via laser levelling to ensure the base is exactly level;
- A diversion berm/bank/drain should be built upslope of the trench. This will reduce run on. A design sketch is provided at **Appendix D**.

7.0 RECOMMENDATIONS

As per the 'On-Site Sewerage Management for Single Households' (1998) publication, stakeholders should be aware that all on site systems and components have a finite life and at some point will require replacement. Septic tanks and AWTs' generally require replacement every 25 years, whereas effluent disposal systems can have an expected life between 5-15 years. The owner is encouraged to obtain a copy of the NSW Government "The Easy Septic Guide" (2000) available from - <https://www.olg.nsw.gov.au/wp-content/uploads/Easy-septic-guide.pdf>

As stated in AS1547-2012 section 5.5.3.4, a reserve application area of similar size to the current design should be considered as part of the risk management process to be available on a site for expansion or for resting of the land application system.

The option provided in this report is a primary treatment septic fed into absorption beds. This is to be designed to accept the discharge from the wastewater treatment unit and it convey it securely and evenly to the land application area. The aim is to ensure uniform distribution of the effluent over the design area to help achieve effective aerobic/anaerobic decomposition within the soil. Typical design sketches for a bed/trench system as per AS 1547:2012 and *Design and Installation of On Site Wastewater Treatment* (2023) are provided at **Appendix D**.

Installation instructions shall be provided by the manufacturer or designer. Barnson will not be liable for the incorrect installation and/or construction of the system. Installation should be in accordance with the prescriptions within AS 1547:2012.

Barnson has not verified the accuracy or completeness of this data, except otherwise stated in this report. The recommendations for the proposed system as suggested in this report are based on historical data obtained for the area. Barnson will not be liable in relation to incorrect recommendations should any information provided by the client be incorrect or have been concealed, withheld, misrepresented or otherwise not fully disclosed.

The accuracy of geotechnical engineering advice provided in this report may be limited by unobserved variations in ground conditions across the site in areas between and beyond test locations and by any restrictions in the sampling and testing which was able to be carried out, as well as by the amount of data that could be collected given the project and site constraints.

These factors may lead to the possibility that actual ground conditions and materials behaviour observed at the test locations may differ from those which may be encountered elsewhere on the site.

If the sub-surface conditions are found to differ from those described in this report, we should be informed immediately to evaluate whether recommendations should be reviewed and amended if necessary.

Please do not hesitate to contact the undersigned if you have enquiries regarding this report.

Yours Faithfully

Reviewed By

Jeremy Wiatkowski
Laboratory Technician

Andrew Ruming
Environmental Geologist

barnson.

APPENDIX A

Water Balance

Barnson Job No	46937-ER01_A	
Location :	Windeyer	

Design Wastewater Flow	Q	l/day	240
Design Loading Rate	R	mm/day	10

Climate Zone	3 C	As per Soil Landscapes of Dubbo 1:250 000 Dropbox
--------------	-----	--

1	2	3	4	5	6	7	8	9	
Month	Pan evap E (mm)	Evapo Transpiration Et (ET=0.75E)mm	Rainfall R (mm)	Retained Rainfall Rr (Rr=0.75R) mm	DLR per Month (mm)	Disposal Rate (3-5+6) mm	Effluent applied per mo (L)	Size of Area (8/7) m ²	Days in Month
Jan	229	171.75	94	70.5	310	411.25	7440	18.09118541	31
Feb	178	133.5	86	64.5	290	359	6960	19.38718663	29
Mar	155	116.25	76	57	310	369.25	7440	20.14895058	31
Apr	104	78	64	48	300	330	7200	21.81818182	30
May	51	38.25	70	52.5	310	295.75	7440	25.15638208	31
Jun	46	34.5	75	56.25	300	278.25	7200	25.87601078	30
Jul	41	30.75	60	45	310	295.75	7440	25.15638208	31
Aug	58	43.5	66	49.5	310	304	7440	24.47368421	31
Sep	89	66.75	60	45	300	321.75	7200	22.37762238	30
Oct	130	97.5	81	60.75	310	346.75	7440	21.45638068	31
Nov	165	123.75	78	58.5	300	365.25	7200	19.71252567	30
Dec	229	171.75	96	72	310	409.75	7440	18.15741306	31
Mean area								21.8m ²	

Month	First trial area	Application rate	Disposal rate	mm	Increase in Depth of Stored Effluent	Depth of Effluent for Mo	Increase in Depth of Effluent	Computed	Reset if Et<0	Equiv Storage
Dec	24m ²	310	409.75	-99.75	-332.5	0	-332.5	-332.5	0	0
Jan		310	411.25	-101.25	-337.5	0	-337.5	-337.5	0	0
Feb		290	359	-69	-230	0	-230.0	-230.0	0	0
Mar		310	369.25	-59.25	-197.5	0	-197.5	-197.5	0	0
Apr		300	330	-30	-100	0	-100.0	-100.0	0	0
May		310	295.75	14.25	47.5	0	47.5	47.5	47.5	1140
Jun		300	278.25	21.75	72.5	47.5	120.0	120.0	120	2880
Jul		310	295.75	14.25	47.5	120	167.5	167.5	167.5	4020
Aug		310	304	6	20	167.5	187.5	187.5	187.5	4500
Sep		300	321.75	-21.75	-72.5	187.5	115.0	115.0	115	2760
Oct		310	346.75	-36.75	-122.5	115	-7.5	-7.5	0	0
Nov		300	365.25	-65.25	-217.5	0	-217.5	-217.5	0	0
Dec		310	409.75	-99.75	-332.5	0	-332.5	-332.5	0	0
Jan		310	411.25	-101.25	-337.5	0	-337.5	-337.5	0	0
Feb		290	359	-69	-230	0	-230.0	-230.0	0	0
Mar		310	369.25	-59.25	-197.5	0	-197.5	-197.5	0	0
Apr		300	330	-30	-100	0	-100.0	-100.0	0	0
May		310	295.75	14.25	47.5	0	47.5	47.5	47.5	1140

Estimated area of effluent drainfield	24m ²
Maximum depth of stored effluent (must not exceed 350mm)	187.5mm
Bed/Trench dimensions	2000mm
Length of bed/trench required	12m
<20m lengths of bed/trench	0.6



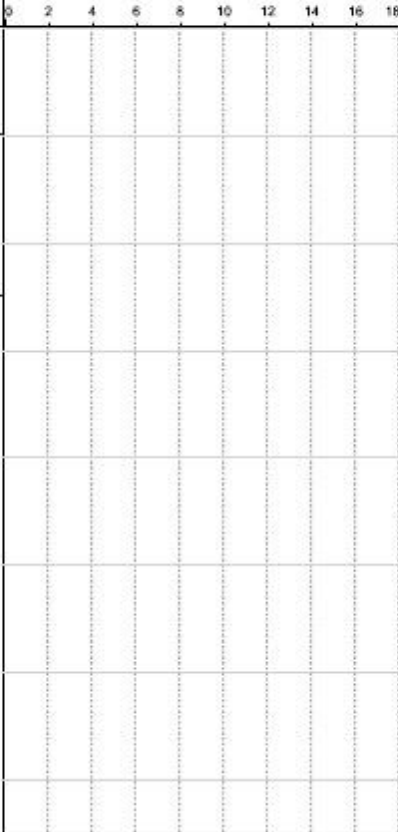


Trench Depth	450
--------------	-----



APPENDIX B

Borehole Logs & Laboratory Results

Latitude : Location : 1808 Windeyer Road, Windeyer NSW Job Number : 46937
Longitude : Logged By : David Brown Client : Jorge Diaz
Total Depth : 1.5 m Date : 07/03/2025 Project : Septic Design

Drilling Method	Depth (m)	Graphic Log	Classification Code	Material Description	DCP Graph	Samples
						Disturbed sample
			TS	Topsoil Sandy SILT very stiff, low plasticity, pale grey-brown, fine grained sand, w < pl.		
	0.2		ML	Alluvial Sandy SILT very stiff to hard, low plasticity, pale brown, fine grained sand, trace fine sized gravel, w < pl.		
	0.5		ML	Alluvial Clayey to sandy SILT hard, low plasticity, pale brown, fine grained sand, with fine to medium sized gravel, w < pl.		
	1					LS=3.5%, PI=12%
				Borehole 1 Terminated at 1.5m		

Material Test Report

Report Number: 46937-1
Issue Number: 1
Date Issued: 17/03/2025
Client: Jorge Diaz
1808 Windeyer Road, Windeyer NSW 2850
Contact: Jorge Diaz
Project Number: 46937
Project Name: Septic Design
Project Location: 1808 Windeyer Road, Windeyer NSW
Work Request: 12040
Sample Number: D25-12040A
Date Sampled: 07/03/2025
Dates Tested: 07/03/2025 - 13/03/2025
Sampling Method: AS 1289.1.2.1 6.5.3 - Power auger drilling
Site Selection: Selected by Client
Sample Location: Borehole 1, Depth: 800mm
Material: Brown Clayey Sandy SILT With Gravel

barnson.

Barnson Pty Ltd

Dubbo Laboratory

16 L Yarrandale Road Dubbo NSW 2830

Phone: 1300 BARNSON

Email: jeremy@barnson.com.au

Accredited for compliance with ISO/IEC 17025 - Testing



Approved Signatory: Jeremy Wiatkowski

Geotechnical Technician

NATA Accredited Laboratory Number: 9605

Atterberg Limit (AS1289 3.1.2 & 3.2.1 & 3.3.1)		Min	Max
Sample History	Oven Dried		
Preparation Method	Dry Sieve		
Liquid Limit (%)	26		
Plastic Limit (%)	14		
Plasticity Index (%)	12		

Linear Shrinkage (AS1289 3.4.1)		Min	Max
Moisture Condition Determined By	AS 1289.3.1.2		
Linear Shrinkage (%)	3.5		
Cracking Crumbling Curling	None		

Emerson Class Number of a Soil (AS 1289 3.8.1)		Min	Max
Emerson Class	6		
Soil Description	Brown Clayey Sandy SILT With Gravel		
Nature of Water	Distilled		
Temperature of Water (°C)	30		



APPENDIX C

Site Setback Requirements

TABLE R1
GUIDELINES FOR HORIZONTAL AND VERTICAL SETBACK DISTANCES
(to be used in conjunction with Table R2)

Site feature	Setback distance range (m) (See Note 1)	Site constraint items of specific concern (from Table R2) (see Note 1)
	<i>Horizontal setback distance (m)</i>	
Property boundary	1.5 – 50 (see Note 2)	A, D, J
Buildings/houses	2.0 – > 6 (see Note 3)	A, D, J
Surface water (see Note 4)	15 – 100	A, B, D, E, F, G, J
Bore, well (see Notes 5 and 6)	15 – 50	A, C, H, J
Recreational areas (Children's play areas, swimming pools and so on) (see Note 7)	3 – 15 (see Notes 8 and 9)	A, E, J
In-ground water tank	4 – 15 (see Note 10)	A, E, J
Retaining wall and Embankments, escarpments, cuttings (see Note 11)	3.0 m or 45° angle from toe of wall (whichever is greatest)	D, G, H
	<i>Vertical setback distance (m)</i>	
Groundwater (see Notes 5, 6, and 12)	0.6 – > 1.5	A, C, F, H, I, J
Hardpan or bedrock	0.5 – ≥ 1.5	A, C, J
NOTES: 1 The overall setback distance should be commensurate with the level of risk to public health and the environment. For example, the maximum setback distance should be adopted where site/system features are on the high end of the constraint scale. The setback distance should be based on an evaluation of the constraint items and corresponding sensitive features in Table R2 and how these interact to provide a pathway or barrier for wastewater movement. 2 Subject to local regulatory rules and design by a suitably qualified and experienced person, the separation of a drip line system from an upslope boundary, for slopes greater than 5%, may be reduced to 0.5 m.		

TABLE R1
GUIDELINES FOR HORIZONTAL AND VERTICAL SETBACK DISTANCES
(to be used in conjunction with Table R2) (continued)

3	Setback distances of less than 3 m from houses are appropriate only where a drip irrigation land application system is being used with low design irrigation rates, where shallow subsurface systems are being used with equivalent low areal loading rates, where the risk of reducing the bearing capacity of the foundation or damaging the structure is low, or where an effective barrier (designed by a suitably qualified and experienced person) can be installed. This may require consent from the regulatory authority.
4	Setback distance from surface water is defined as the areal edge of the land application system to the edge of the water. Where land application areas are planned in a water supply catchment, advice on adequate buffer distances should be sought from the relevant water authority and a hydrogeologist. Surface water, in this case, refers to any fresh water or geothermal water in a river, lake, stream, or wetland that may be permanently or intermittently flowing. Surface water also includes water in the coastal marine area and water in man-made drains, channels, and dams unless these are to specifically divert surface water away from the land application area. Surface water excludes any water in a pipe or tank.
5	Highly permeable stony soils and gravel aquifers potentially allow microorganisms to be readily transported up to hundreds of metres down the gradient of an on-site system (see R3, Table 1 in Pang et al. 2005). Maximum setback distances are recommended where site constraints are identified at the high scale for items A, C, and H. For reading and guidance on setback distances in highly permeable soils and coarse-grained aquifers see R3. As microbial removal is not linear with distance, data extrapolation of experiments should not be relied upon unless the data has been verified in the field. Advice on adequate buffer distances should be sought from the relevant water authority and a hydrogeologist.
6	Setback distances from water supply bores should be reviewed on a case-by-case basis. Distances can depend on many factors including soil type, rainfall, depth and casing of bore, direction of groundwater flow, type of microorganisms, existing quality of receiving waters, and resource value of waters.
7	Where effluent is applied to the surface by covered drip or spray irrigation, the maximum value is recommended.
8	In the case of subsurface application of primary treated effluent by LPED irrigation, the upper value is recommended.
9	In the case of surface spray, the setback distances are based on a spray plume with a diameter not exceeding 2 m or a plume height not exceeding 0.5 m above finished surface level. The potential for aerosols being carried by the wind also needs to be taken into account.
10	It is recommended that land application of primary treated effluent be down gradient of in-ground water tanks.
11	When determining minimum distances from retaining walls, embankments, or cut slopes, the type of land application system, soil types, and soil layering should also be taken into account to avoid wastewater collecting in the subsoil drains or seepage through cuts and embankments. Where these situations occur setback clearances may need to be increased. In areas where slope stability is of concern, advice from a suitably qualified and experienced person may be required.
12	Groundwater setback distance (depth) assumes unsaturated flow and is defined as the vertical distance from the base of the land application systems to the highest seasonal water table level. To minimise potential for adverse impacts on groundwater quality, minimum setback distances should ensure unsaturated, aerobic conditions in the soil. These minimum depths will vary depending on the scale of site constraints identified in Table R2. Where groundwater setback is insufficient, the ground level can be raised by importing suitable topsoil and improving effluent treatment. The regulatory authority should make the final decision in this instance. (See also the guidance on soil depth and groundwater clearance in Tables K1 and K2.)

TABLE R2
SITE CONSTRAINT SCALE FOR DEVELOPMENT OF SETBACK DISTANCES

(used as a guide in determining appropriate setback distances from ranges given in Table R1)

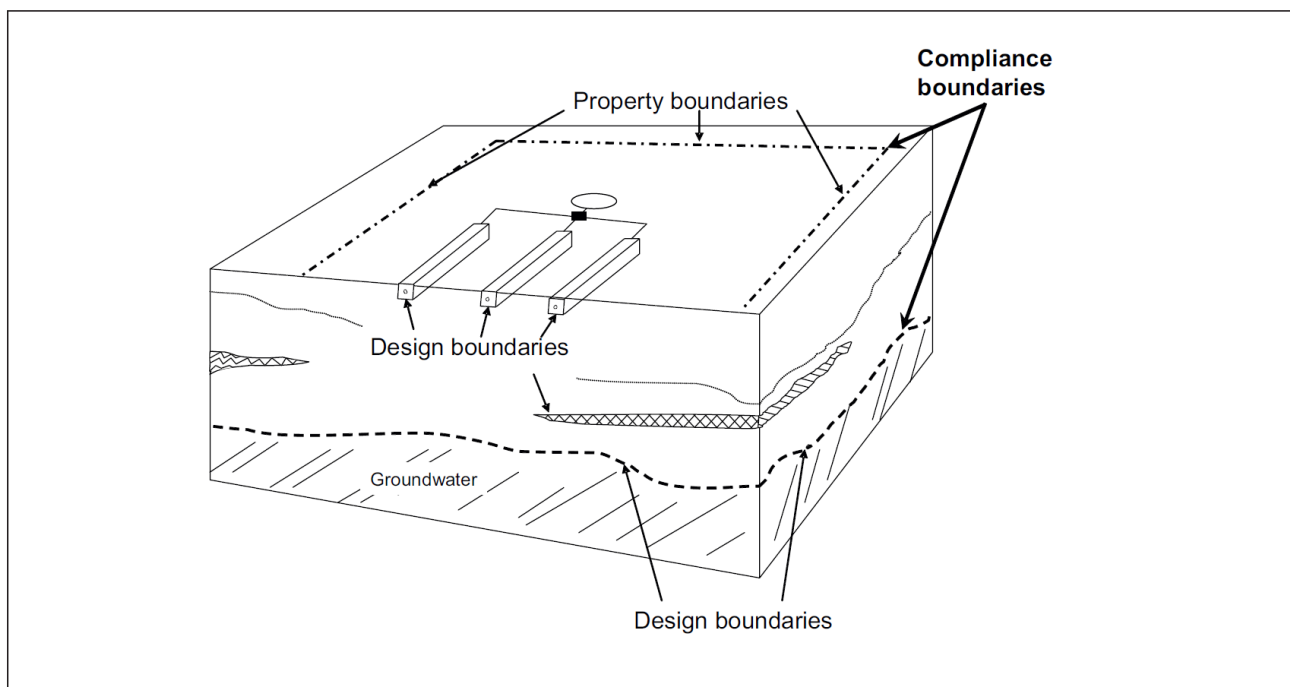
Item	Site/system feature	Constraint scale (see Note 1)		Sensitive features
		LOWER	HIGHER	
		Examples of constraint factors (see Note 2)		
A	Microbial quality of effluent (see Note 3)	Effluent quality consistently producing ≤ 10 cfu/100 mL <i>E. coli</i> (secondary treated effluent with disinfection)	Effluent quality consistently producing $\geq 10^6$ cfu/100 mL <i>E. coli</i> (for example, primary treated effluent)	Groundwater and surface pollution hazard, public health hazard
B	Surface water (see Note 4)	Category 1 to 3 soils (see Note 5) no surface water down gradient within > 100 m, low rainfall area	Category 4 to 6 soils, permanent surface water <50 m down gradient, high rainfall area, high resource/environmental value (see Note 6)	Surface water pollution hazard for low permeable soils, low lying or poorly draining areas
C	Groundwater	Category 5 and 6 soils, low resource/environmental value	Category 1 and 2 soils, gravel aquifers, high resource/environmental value	Groundwater pollution hazard
D	Slope	0 – 6% (surface effluent application) 0 – 10% (subsurface effluent application)	> 10% (surface effluent application), > 30% subsurface effluent application	Off-site export of effluent, erosion
E	Position of land application area in landscape (see Note 6).	Downgradient of surface water, property boundary, recreational area	Upgradient of surface water, property boundary, recreational area	Surface water pollution hazard, off-site export of effluent
F	Drainage	Category 1 and 2 soils, gently sloping area	Category 6 soils, sites with visible seepage, moisture tolerant vegetation, low lying area	Groundwater pollution hazard
G	Flood potential	Above 1 in 20 year flood contour	Below 1 in 20 year flood contour	Off-site export of effluent, system failure, mechanical faults
H	Geology and soils	Category 3 and 4 soils, low porous regolith, deep, uniform soils	Category 1 and 6 soils, fractured rock, gravel aquifers, highly porous regolith	Groundwater pollution hazard for porous regolith and permeable soils
I	Landform	Hill crests, convex side slopes, and plains	Drainage plains and incise channels	Groundwater pollution hazard, resurfacing hazard
J	Application method	Drip irrigation or subsurface application of effluent	Surface/above ground application of effluent	Off-site export of effluent, surface water pollution

NOTES:

- Scale shows the level of constraint to siting an on-site system due to the constraints identified by SSE evaluator or regulatory authority. See Figures R1 and R2 for examples of on-site system design boundaries and possible site constraints.
- Examples of typical siting constraint factors that may be identified either by SSE evaluator or regulatory authority. Site constraints are not limited to this table. Other site constraints may be identified and taken into consideration when determining setback distances.

TABLE R2
SITE CONSTRAINT SCALE FOR DEVELOPMENT OF SETBACK DISTANCES
(used as a guide in determining appropriate setback distances from ranges given
in Table R1) (continued)

- | | |
|---|--|
| 3 | The level of microbial removal for any on-site treatment system needs to be determined and it should be assumed that unless disinfection is reliably used then the microbial concentrations will be similar to primary treatment. Low risk microbial quality value is based on the values given in ARC (2004), ANZECC and ARMCANZ (2000), and EPA Victoria (<i>Guidelines for environmental management: Use of reclaimed water</i> 2003). |
| 4 | Surface water, in this case, refers to any fresh water or geothermal water in a river, lake, stream, or wetland that may be permanently or intermittently flowing. Surface water also includes water in the coastal marine area and water in man-made drains, channels, and dams unless these are to specifically divert surface water away from the land application area. Surface water excludes any water in a pipe or tank. |
| 5 | The soil categories 1 to 6 are described in Table 5.1. Surface water or groundwater that has high resource value may include potable (human or animal) water supplies, bores, wells, and water used for recreational purposes. Surface water or groundwater of high environmental value include undisturbed or slightly disturbed aquatic ecosystems as described in ANZECC and ARMCANZ (2000). |
| 6 | The regulatory authority may reduce or increase setback distances at their discretion based on the distances of the land application up or downgradient of sensitive receptors. |



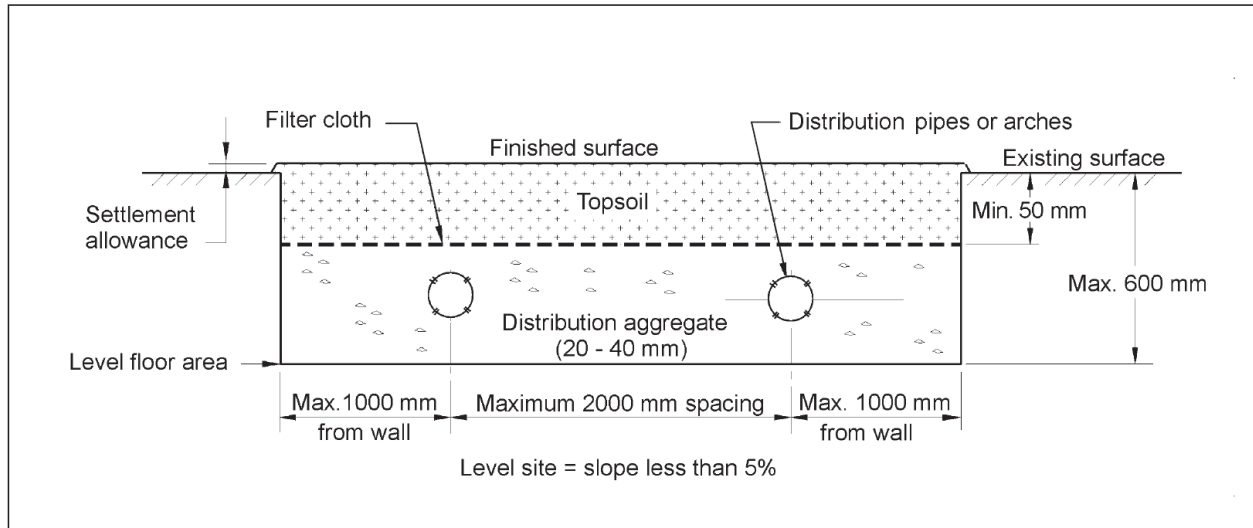
(Adapted from USEPA 2002)

FIGURE R1 EXAMPLE OF DESIGN AND COMPLIANCE BOUNDARIES FOR APPLICATION OF SETBACK DISTANCES FOR A SOIL ABSORPTION SYSTEM



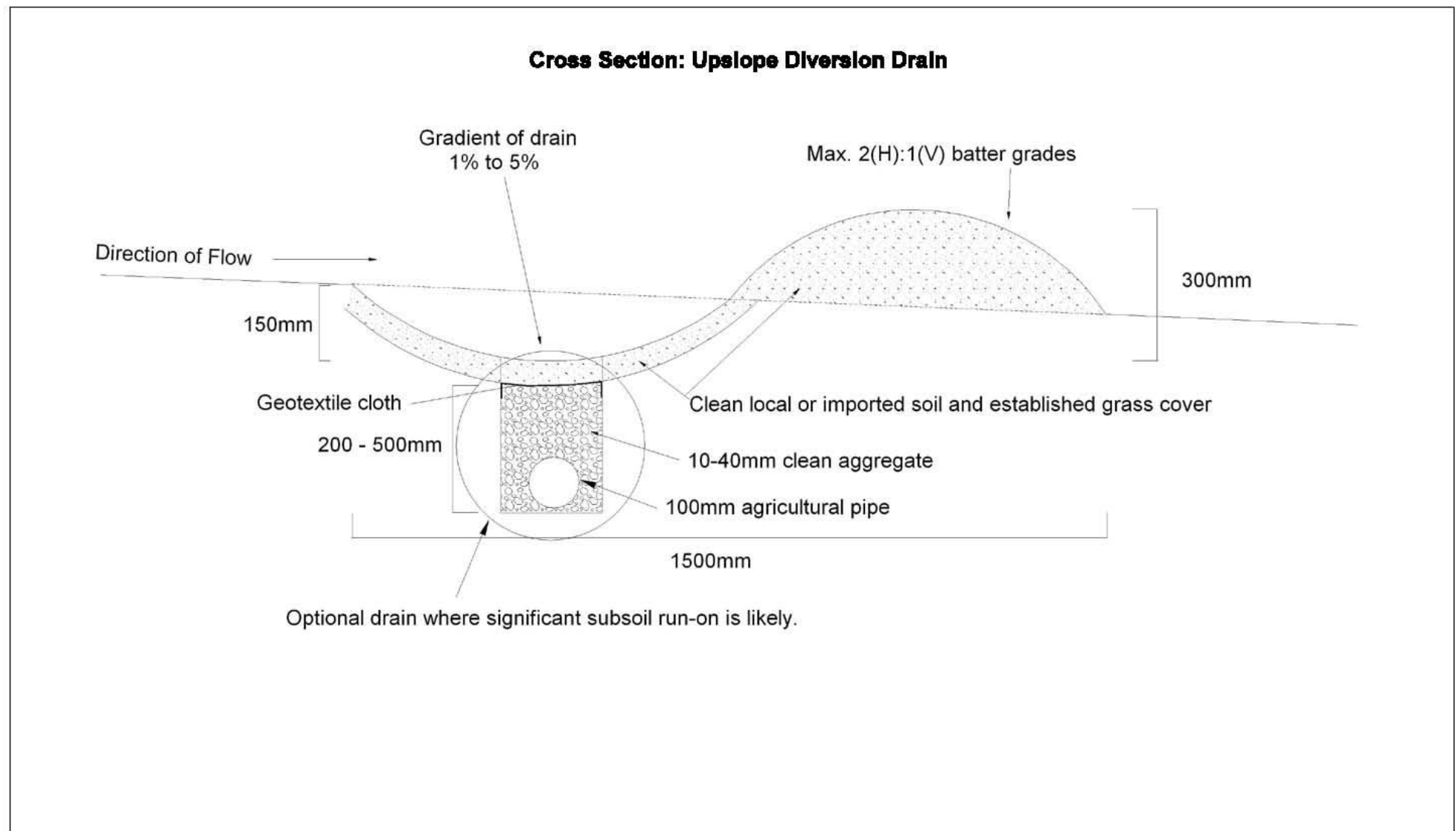
APPENDIX D

Absorption Bed Concept Plans



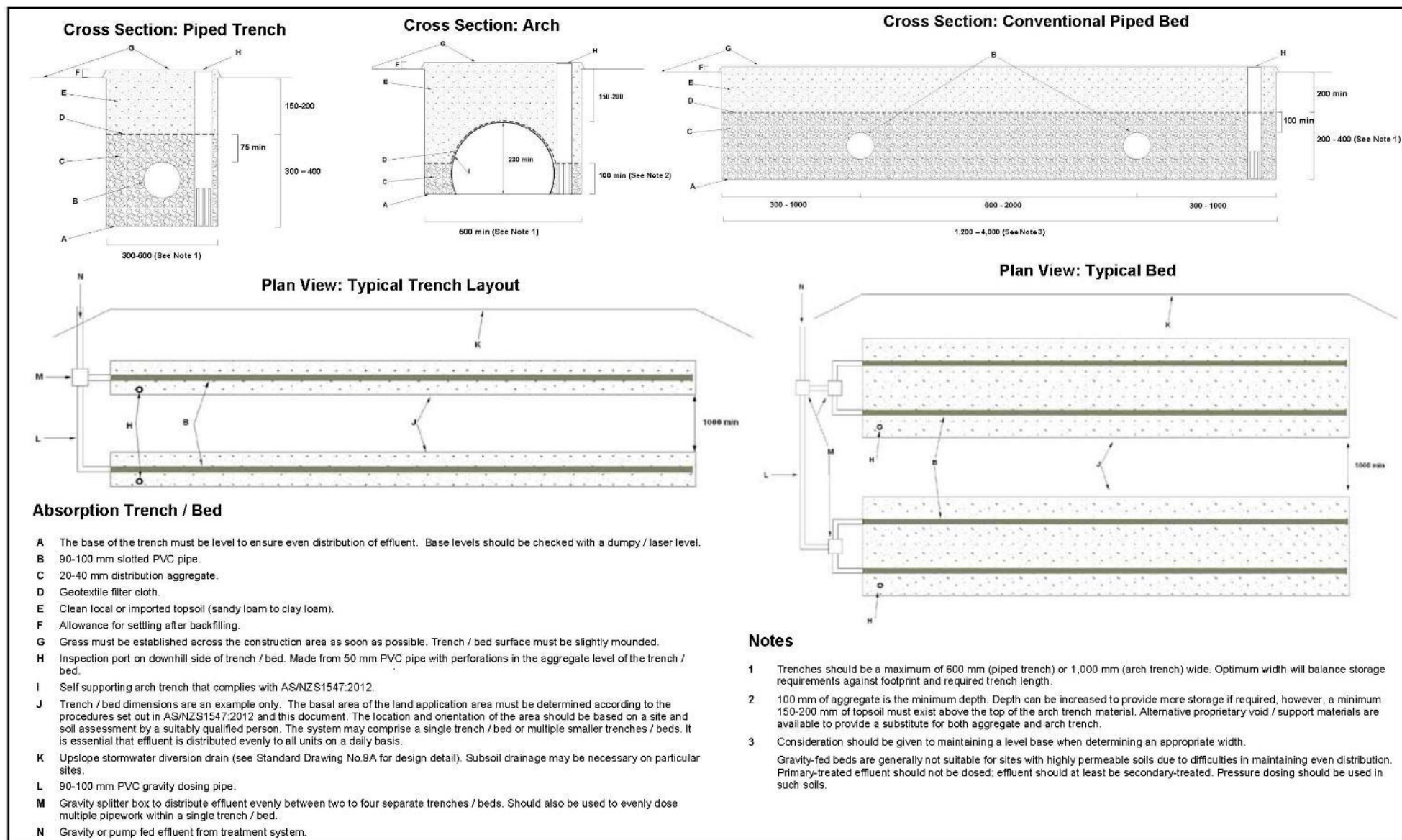
NOTE: LPED lines can be used instead of distribution pipes when dose loading effluent into beds.

FIGURE L5 CONVENTIONAL BED



Standard Drawing 10A - Upslope Diversion Drain

(not to scale)



Standard Drawing 10B - Absorption Trench / Bed
(not to scale)

barnson.

APPENDIX E

List of Plates



Plate 1 – Overview of proposed site



Plate 2 – Overview of proposed site