

Unit 4 / 108-110 Market Street Mudgee, NSW 2850

1300 BARNSON (1300 227 676)

🤀 generalenquiry@barnson.com.au

to. Geoff Spice 433 Kaludabah Road Piambong NSW 2850 <mark>date.</mark> 4.04.2023

reference. 41215-SL01_A

Dear Geoff,

Re: Kaludabah Heights at 433 Kaludabah Road, Piambong NSW 2850 Internal Fit-Out for Habitable Shed – Building Information Certificate (BIC) Structural Inspection

I hereby certify that Mr. Sam Rochester, a Structural Engineer, visited the above address on Thursday 30th March 2023. The purpose of the inspection was to review and certify the internal works completed within the existing shed structure to obtain a Building Information Certificate (BIC) with Mid-Western Regional Council.

The client advised that the shed was constructed approximately eight years prior to the inspection and was modified in August of 2022 for use as a habitable structure. The client provided the attached design drawings produced by the shed supplier, Wide Span Sheds, along with photographs taken during the internal fit-out. The drawings have been stamped during construction by Whitehall Building Certifiers indicating that the shed is adequate for use as a Class 10a building as per the design drawings.

Further evidence that the shed was constructed in accordance with the design drawings was collected during the inspection. The steel framing of the original shed was inspected directly by removing some of the roof and wall sheets. The slab was confirmed to be minimum 100mm thick with a plastic membrane installed underneath and mass concrete piers located below each column. This matches the specification provided for Class A, S or M sites. A geotechnical investigation was completed by Barnson at the site of the shed in 2015. The report produced following this investigation, 23581-GR01_A, advises that a site classification of M-D should be adopted.

The stamped drawings and observations made during the inspection confirm that the original shed was constructed in accordance with the design drawings and is therefore structurally adequate for use as a Class 10a building. The internal fit-out has been completed in order to convert the building



to a habitable structure or Class 1a building. Based on the provided photographs the steel and timber framing installed as part of the internal fit-out are in accordance with the NASH standard and AS1684.2 respectively. The slab was designed for a Class 10a building and specification for the type of slab used is not provided in *AS2870:2011 – Residential slabs and footings*. Although the slab has not been designed for residential purposes, we advise that it is structurally adequate to support the floor loading for a self-contained dwelling. It should be noted however that due to the low stiffness of the slab, cracking should be expected to brittle installations (i.e., plasterboard lining).

Some additions not included on the original design drawings were noted. A steel framed verandah was attached to the South-West, South-East and North-East side of the building. A 100mm thick concrete slab was installed below this verandah on the North-East side and along a partial length of the South-East side. The steel-framed verandah was noted to have C150 rafters spanning maximum 3m to a C200 verandah beam, which is supported over 100SHS columns embedded into concrete piers.

Photographs of the structure as provided by the client during the fit-out and taken during the inspection are included in Appendix B.

Upon analysis, the structure has been checked in accordance with accepted engineering principles and has been found to generally be structurally adequate to the below Australian Standards and design parameters:

1. Loading:

General principles of loading calculation and loading combinations to Australian Loading Code AS1170.0-2002 and other relevant codes as below:

- a. Dead Loads:
 - i. Roof: Weight of steel sheeted and cold-formed steel framed roof with plasterboard ceiling lining, 0.3kPa.
 - ii. Roof: Weight of solar panels, maximum 0.2kPa.
 - iii. Framing: Self-weight of steel wall sheeting, cold-formed steel framing elements and internal wall lining, 0.4kN/m².
 - iv. Floor: Self-weight of reinforced concrete floor slab, 25kN/m³.
- b. Live Loads:
 - i. Roof: Maintenance load of 1.8/A + 0.12kPa (min 0.25kPa), or 1.4kN point loading as per AS1170.1-2002.
 - ii. Floor: Imposed action for general areas of self-contained dwellings, 1.5kPa distributed action or 1.8kN point loading as per AS1170.1-2002.
- c. Wind Loads:
 - i. Wind Class N3 to AS4055-2012.



2. Design Standards:

- a. AS/NZS 1170.0:2002 Structural Design Actions, General Principles
- b. AS/NZS 1170.1:2002 Structural Design Actions, Permanent Imposed and Other Actions
- c. AS/NZS 1170.2:2011 Wind Actions
- d. AS/NZS 1684.2:2010 Residential Timber Framed Construction
- e. AS4100:1998 Steel Structures

We advise the structure complies with the above standards as is suitable for use as a habitable structure.

If you have any further enquiries regarding this matter, please contact the undersigned.

Yours faithfully,

BARNSON PTY LTD



Luke Morris BE ME FIEAust CPEng (Reg) Director

Encl:

- Design Drawings Wide Span Sheds
- Site Photographs





APPENDIX A Building Plans/Documentation

Design Information Sheet

Building Details

Item	Design Value					
Building Description	Portal Design Steel Frame and Steel Clad Building					
Job Reference #	WSS160069					
Site Address	433 Kaludbah Rd PIAMBONG NSW 2850					
Building Classification	Class 10					
Effective Design Height	3.18 m					
	Main Building	Left Leanto	Right Leanto			
Length	12m	NA	NA			
Bay Size	4 eventy spaced	NA	NA			
Span	7m	NA	NA			
Roof Pitch	06 degrees	NA	NA			

Pressure Coefficients

Item	Design Value	Reference		
Internal Pressure coefficient CPI	0.7	AS/NZS 1170.2 Clause 5.3		
Windward Wall (External Across)	0.7	AS/NZS 1170.2 Table 5.2 (A)		
Windward Rafter (External Across)	-0.9, -0.4	AS/NZS 1170.2 Table 5.3 (B)		
Leeward Rafter (External Across)	-0.47	AS/NZS 1170.2 Table 5.3 (C)		
Leeward Wall (External Across)	-0.5	AS/NZS 1170.2 Table 5.2 (B)		
Side Wall (External Along)	-0.65	AS/NZS 1170.2 Table 5.2 (C)		
Rafters (External Along)	-0.9, -0.4	AS/NZS 1170.2 Table 5.3 (A)		
Local Pressures Applied	Yes	AS/NZS 1170.2 Table 5.6		

Site Classification

Item	N	NE	E	SE	S	SW	W	NV	v
Wind Region	A1	2							
Importance level	2	46					4		
Terrain Category	2	2	2	2.5	2.5	2	2.5	2	
Shielding Multiplier (Ms)	1	0.9	0.95	1	1	1	0.95	1	
Topographic Multiplier (Mt)	1	1	1	1	1	1	1	1	
Wind Direction Multiplier (Md)	0.9	0.8	0.8	0.8	0.85	0.95	1	0.9	5
Design Wind Speed	39 n	n∕s						Ī	WHITEHALL BUILDING
Snowload Factor	0kPa							CERTIFIERS	
Seismic Factor (Z)	NA		123-14			S.			O E I C I I I I I I I I I I I I I I I I I
									COMPLYING DEVELOPMENT CERTIFICATE NO

Trimangle Root	Purin	100mm	then-	NG	7/6
Wat Sheeting - twister -	Ratter	₿ XE	11	WHITEHALL BUILDI CERTIFIERS COMPLYING DEVELOPA	CERTIFICATE NO
				SICKIERS ENCIREERING PTV LTD ACK 075 007 144	



















MATERIAL SPECIFICATIONS

eters. Pier sizes nominated are suitable for Class A, S or M sites. Refer to General Notes.

		Building C	Imensions		-		
Categories	Span	Length	Plich	Height	Gridija) Pontal(a)	
Main Building	7	12	06	3	A-B	1-5	
		Postal Fran	ne Bernerd				
Out / Doutel Marsher	T	1	2	3	4	5	
CHUTFORD HORSE	A	C15012	C20015	C20015	C20015	C15012	
	Discreter	0.3	0.45	0.45	0.45	0.3	
	Dept	0.75	0.8	0.8	0.8	0.75	
	8	C15012	2020015	2C20015	2C2001	5 C15012	
0	Diameter	0.3	0.45	0.45	0.45	0.3	
-	Death	0.75	12	1.2	1.2	0.75	
Dathers	A-B	C15012	C20015	C20015	C20015	C15012	
Cont Made and Marrie	5	C15019	-	-	F	C15019	
CO THE POSICIO	Diamater	0.3	-	-	-	0.3	
	Death	0.75	-	-	-	0.75	
New Design	4.8	-	C10012	C10012	C10012	-	
VN66 EF8K086	R.A		C10012	C10012	C10012	-	
		S. Same		Buy Sectio	n Bernenti		
Grid / Bary Nur	nber			1		2	-
Per Write				3			3
Boot Purlins		A-B		210010		10010	210010
Roof Purlin Specing (End)		A-8		1.408		405	1,408
Roof Purlin Specing (In	ternal Spane	A-B		1,408		408	1,408
Eme Girt		A 2		210010		10010	210010
Eeve Purlin		8		2C10010		C10010	2010010
Side Cirts		A		210010		10010	210010
		8					1
Side Cirts Specing (End)		A		1.109 1		109	1.169
		8		0.11		.11	0.11
Side Girls Specing (Internal)		4		1.169	1,169		1,169
		B		0.11	D	.11	0.11
		8 d B	by Bection	Elements			
Grid / Portel Numbe	*			1		5	
End Girls	A-Z	A-Z		0010		210010	
	Z-8	1010	Z	10010		210010	
End Girls Spacing (End	A-Z	1.		.385		1.385	
	Z-8			385		1.385	
End Gints Specing (Inter	mel)A-Z		1.3				
Z-B			1:	385		1.385	

Colour Product Zhoshuma Thimdad 0.42 BMF (0.47TCT) Zhoshuma Thimdad 0.42 BMF (0.47TCT) Calegories Roof Well

Purchaser Name: Geolf Spice

She Address: 433 Kaludbeh Rd PWMBONG NSW Australia 2850

Drawing # WSS160069 - 4

Print Date: 21/01/16

Specification Sheet Page 1of 1

Z10010 1.406 1.406 210010 2C10010 210010

1.169 0.11 1.169 0.11

Seller: Wide Span Sheds Steeb: Piy Ltd Phone: 07 5657 8868 Fax: 07 5657 8889 Emeil: admin@ahede.com.au

SHOWERS ENGINEERING PTYLTD ACR 075 007 144 ME ANAL (Registered NPER Structure) & Cvil) 321787 ADD (IPRED No. 1547, NO. ECCH (2), TKS (CCH600+), NT: 46926ES, Preding Productivel Structure) & Cvil Digineer

Signature:

R.J. Showers Date: 21/01/16

Sé

BPB 0439

COMPLYING DEVELOPMENT

CERTIFICATE NO..... PCA

WHITEHALL BUILDING

CERTIFIERS



GENERAL NOTES

These documents show the general arrangement of the building and include some items not supplied (refer to the quotation for nomination of all items to be provided). All items not nominated therein shall be supplied and installed by others.

DESIGN CRITERIA

These building plans have been prepared to comply with the standards nominated in the ensincer's letter and itemised details in the attached Design Information Sheet.

The plans provided here are the latest at the time of print. Earlier plans provided may have become outdated due to engineering changes and should not be used. The plans and drawings are extensive and give all the information needed for a competent person to erect the building. The building is not designed to stand up by itself when it is partially complete. Consequently, construction bracing is critical during erection.

The owner has been requested to check off the BOM after the building delivery. You should check that you are able to locate all materials nominated in the BOM. You should also confirm that the length and size (including thickness), nominated in the BOM is what has been provided. Any missing items are the responsibility of the client once correct delivery has been confirmed as per Terms and Conditions of Sale.

ADDITIONAL DOCUMENTATION TO BE SUPPLIED BY PURCHASER/OWNER

The Purchaser/Owner is responsible for:

*Provision of Soils Report for the site and in the building area on which the building is to he erected

*Site/Drainage Plans

*Any other plans not covered by these engineering plans requested by the local Council or the authority

BUILDING CONSTRUCTION REQUIREMENTS

The Purchaser/Owner is to be ensured that all building construction is carried out in accordance with the Plans, the Construction Manual and the Bill of Materials (BOM).

SLAB AND PIER DETAILS - GENERAL

* Where columns or end wall mullions have been removed, piers are not required * End wall multion spacing may move due to location of openings or doors. Check layout plan, and relocate piers as required.

Commente Slah

* Design covers sites with a minimum of 100kPa safe bearing capacity and of soil classifications of A, S, M, H1 or H2 for a class 10 building. Other than this, owner to provide slab and pier design details.

* Designs are in accordance with AS 3600:2009

* All concrete to be in accordance with AS 3600:2009. Minimum 25 Mpa, with 80mm shimo

* Concrete should be cured for 7 days before commencing construction of the building. * Refer to connection details.

Saw construction joints to be 25mm deep x 5mm wide. Saw cuttings shall take place no later than 24 hours after pouring. Saw construction joints to be placed at a maximum spacing of 6.3m (in both the length and the span). Care should be taken to avoid construction cuts intersecting where any foung to the slab is to be made.

For Class A, S or M Shes

* Slab thickness to be a minimum of 100mm with SL 72 mesh and 40mm top cover.

* The minimum size of Piers under the columns and End Wall Mullions are nominated below. When the slab and piers are poured as one pour, the depth of the pier is to the top of the slah

C15012.C15019 - 300mm dia x 750mm deep, centered to the C Section C20015 - 450mm dia x 800mm deep, centered to the C Section

2C20015 - 450mm dia x 1200mm deep, centered to the C Section

* Pier Reinforcement: for any piers over 1100mm, deformed bar to within 100mm of base and minimum 75mm top cover. Minimum side cover 75mm, maximum 100mm. Rod to be caged horizontally at least twice and at a maximum of 300mm spacing. Tie with a minimum of 6mm diameter case tie. Where pier diameter is less than 450mm diameter, use 4 N12. For diameters equal to and over 450mm, use 4 N16.

For Class HI or H2 Sites

* Slab thickness to be a minimum of 125mm with SL 82 mesh and 40mm top cover.

* Edge beam 400mm deep x 300mm wide with Y12 3 bar Trench Mesh to the perimeter of the building

* Thickening beams 400mm deep by 300mm wide with Y12 3 bar Trench Mesh at a max spacing of 6.2m.

* The minimum size of Piers under the columns and End Wall Mullions are nominated below. When the slab and piers are poured as one pour, the depth of the pier is to the top of the slab

C15012,C15019 - 300mm dia x 1000mm deep, centered to the C Section

C20015 - 450mm dia x 1000mm deep, centered to the C Section

2C20015 - 450mm dia x 1500mm deep, centered to the C Section * Pier Reinforcement: for any piers over 1100mm, deformed bar to within 100mm of base and minimum 75mm top cover. Minimum side cover 75mm, maximum 100mm. Rod to be caged horizontally at least twice and at a maximum of 300mm spacing. Tie with a minimum of 6mm diameter cage tie. Where pier diameter is less than 450mm diameter. use 4 N12. For diameters equal to and over 450mm, use 4 N16.

Ownerste Plers Only

* Pier design covers sites with a minimum of 100kPa safe bearing capacity and of soil classification A, S, M, H1 or H2 for a class 10 building. Other than this, owner to provide elab and nier design details.

* Designs are in accordance with AS 3600:2009

* All concrete to be in accordance with AS 3600:2009. Minimum 25 Mpa, with 80mm shimp

* Piers should be cured for 7 days before commencing construction of the building. * All dimensions are from center of columns.

* Refer to connection details

* Saw construction joints to be 25mm deep x 5mm wide. Saw cuttings shall take place no inter than 24 hours after pouring. Saw construction joints to be placed at a maximum spacing of 6.3m (in both the length and the span). Care should be taken to avoid construction cuts intersecting where any fixing to the slab is to be made.

For Class A. S or M Sites

Piers to be a minimum of

C15012.C15019 - 300mm dia x 750mm deep, centered to the C Section C20015 - 450mm dia x 800mm deep, centered to the C Section 2C20015 - 450mm dia x 1200mm deep, centered to the C Section . Piers 1000mm and under in depth do not require any steel reinforcement. Diers over 1000mm in depth to have a minimum of 4 N16 deformed bar to within 10mm of base and minimum 75mm top cover. Minimum side cover 75mm, maximum 100mm. Rod to be cased horizontally at least twice and at a maximum of 500mm spacing. The with minimum of 6mm diameter cage tie. 5

z 0

Ш

Z

ñ

0

Ш

>

í.

0 ž

0

NIX AT S

OMPL

ŏ 111

L 1

TIFIC/

R

0

94

BoB

à

LU

0

For Class H1 or H2 Sites

Piers to be a minimum of

L C15012.C15019 - 300mm dia x 1000mm deep, centered to the C Section (20015 - 450mm dia x 1000mm deep, centered to the C Section . has 2C20015 - 450mm dia x 1500mm deep, centered to the C Section Piers 1000mm and under in depth do not require any steel reinforcement. Hars 1000mm in depth to have a minimum of 4 N16 deformed bar to within 10 mm dethese and minimum 75mm top cover. Minimum side cover 75mm, maximum 100mm. Rod to be caged horizontally at least twice and at a maximum of 500mm spacing The with a minimum of 6mm diameter cage tie. WH

BRACING NOTES

* Refer to Connection Details.

All bracing strap to be 32mm x 1.6mm G450 * Cross bracing is to be fixed taught and secured with 2 x 14.20 x 22 frame screws at

each end.

· Fly bracing to be fixed to the purlins/girts at all mid portal rafters, columns and end wall mullions. The spacing between fly braces is determined by column/rafter sizes as follows:

C150 - maximum 1800mm specing C200, C250 - maximum 2200mm specing C300 - meximum 2800mm specing C350 - maximum 3400mm spacio

Initial measurement is from the haunch of the column/rafter, and from the rafter for any and wall mullions

* Open bays to have fly bracing fitted to every available girt supporting the header sheets. * All bracing strap ends to be located as close as practical to structural members (columns, rafters, mullions) centerline.

BOLTS

* Unless otherwise nominated, all bolts are grade 4.6 and zinc plated * All tensioned bolts shall be tensioned using the part turn method (refer to AS4100). For

the erector full details are in the construction manual

OTHER MATERIALS NOTES

* All sheeting and flashing screws are class 3, all framing screws are class 3, * All purlin material has Z350 zinc coating with minimum strength of 450MPa.

SHOWERS ENDINEEDING PTVI TO ACN: 075 007 144 Purchaser Name: Geoff Spice ME Auki, (Registered NPER Structural & Chil) 321787 CLD: RPEQ No. 1547; VIC: EC24182; TAS: CC4600H; N.T: 45525ES; Seller: Wide Span Shede Steek Pty Ltd Phone: 07 5657 8888 Practising Protessional Structural & Civil Engineer General Notes Site Address; 433 Kaludbeh Rd PWMBONG NSW Australia 2850 Page 1of 1 Fax: 07 5857 8899 Frond adminifestants com as Struttor R.J. Showers Date: 21/01/18 Print Dale: 21/01/16 Drewlova # WES160069 - 2





APPENDIX B Site Photographs





Figure 1 – Top hat battens installed to support ceiling lining (client supplied)



Figure 2 – Top hat frame installed to support internal wall lining (client supplied)



Figure 3 – Exposed timber framing of partition walls (client supplied)



Figure 4 – Wall and ceiling lining installed for fitout (client supplied)

barnson.



Figure 5 – South-East face of converted shed, open and enclosed section of attached verandah



Figure 6 – North-East side of converted shed



Figure 7 – Verandah attached on South-West side of converted shed



Figure 8 – 100SHS verandah Post embedded into concrete pier



Figure 9 – Framing of attached verandah roof on North-East side of converted shed



Figure 10 – Plastic membrane visible under slab edge