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**Plantation
Pine Products**

Appendix F – Traffic Impact Assessment

David Pavey Pty Ltd trading as

Pavey Consulting Services

Specialising in

Traffic Impact Assessments and Transportation Planning
Road Safety, Traffic Management Plans and Traffic Control Plans
Civil and Structural Design
Project Management and Contract Administration
Mediation and Government Relations

Integrated Transport Assessment

Proposed Quarry

39 Razorback Road Running Stream NSW

22 July 2022 Rev 1

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1. INTRODUCTION

Pavey Consulting Services (PCS) has been commissioned by SpaceUrban Pty. Ltd. On behalf of Plantation Pine Products Australia Pty Ltd (PPPA) to provide an Integrated Transport Assessment (ITA) to support an application for a proposed quarry at 39 Razorback Road, Running Stream, NSW.

PPPA proposes to develop and operate a sand and gravel quarry on the 'Turonfels' property at 39 Razorback Road, Running Stream, approximately 8km south of Ilford within the Mid-Western Regional Council Local Government Area (LGA). The site is located approximately 1km west of the Castlereagh Highway.

Figure 1 shows the location of the quarry.

The quarry would extract up to 200,000 tonnes per annum over a period of up to 30 years and will include access roads, a site office, workshop and weighbridge. The quarry will be progressively rehabilitated to pasture and pine plantation with potential future use of the facilities area for forestry related activities.

This report includes consideration of issued raised in Letter Requirements of SEARS No 1523 dated 2/3/21 and additional requirements set out in Transport for NSW (TfNSW) letter of 1 June 2021.

1 Limits of Report

This report considers the instructions and requirements of our client. Pavey Consulting Services (PCS) has taken care in the preparation of this report, however it neither accepts liability nor responsibility whatsoever in respect of:

- Any use of this report by any third party,
- Any third party whose interests may be affected by any decision made regarding the contents of this report, and/or
- Any conclusion drawn resulting from omission or lack of full disclosure by the client, or the clients' consultants.

2 Basis of Integrated Transport Assessment

This Integrated Transport Assessment (ITA) has been prepared in accordance with the relevant governmental assessment requirements, guidelines and policies, and in consultation with the relevant Government Agencies.

The ITA has been developed in accordance with:

- Austroads Guide to Traffic Management Part 3 Traffic Studies and Analysis.
- Austroads Guide to Traffic Management Part 12 Traffic Impacts of Developments; and
- NSW Roads and Maritime Services (RMS) Guide to Traffic Generating Developments (2002).

The assessment is based on the following general scope for matters to consider in a ITA which is defined by the NSW Roads and Maritime Services (RMS) Guide to Traffic Generating Developments (RTA 2002):

- The existing locality and surrounding land uses,
- The existing road networks,
- Traffic generation characteristics,
- Traffic impacts, and
- A summary of assessed traffic impacts and any traffic mitigation measures proposed.

3 SEARS REQUIREMENTS

As outlined in the SEARs and TfNSW requirements the following aspects are address in this report as they relate to Traffic &Transport issues.

SEARS requirements.

- **Traffic &Transport:**
 - accurate predictions of the road traffic generated by the construction and operation of the development, including a description of the types of vehicles likely to be used for transportation of quarry products,
 - an assessment of potential traffic impacts on the capacity, condition, safety and efficiency of the local and State Road networks, detailing the nature of the traffic generated, transport routes, traffic volumes and potential impacts on local and regional roads,
 - a description of the measures that would be implemented to maintain and/or improve the capacity, efficiency and safety of the road network (particularly the proposed transport routes) over the life of the development, and
 - evidence of any consultation with relevant roads authorities, regarding the establishment of agreed contributions towards road upgrades or maintenance; and
 - a description of access roads, specifically in relation to nearby Crown roads and firetrails.

Issues raised by TfNSW:

- **Project schedule:**
 - Hours and days of work, number of shifts and start and end times, and
 - Transport considerations at each phase and stage of the project, including construction, operation and decommissioning.
- **Traffic volumes:**
 - Existing background traffic,
 - Project-related traffic for each phase or stage of the project, and
 - Projected cumulative traffic at commencement of operation, and a 10-year horizon post-commencement.
- **Traffic characteristics:**
 - Number and ratio of heavy vehicles to light vehicles,
 - Peak times for existing traffic,
 - Peak times for project-related traffic including commuter periods,
 - Proposed hours for transportation and haulage,
 - Interactions between existing and project-related traffic, and
 - A description of all over size and over mass vehicles and the materials to be transported.
- **The origins, destinations and routes:**
 - Commuter (employee and contractor) light vehicles and pool vehicles,
 - Heavy (haulage) vehicles, and
 - Over size and over mass vehicles,
- The impact of traffic generation on the public road network and measures employed to ensure traffic efficiency and road safety during construction, operation and decommissioning of the project,
- The need for improvements to the road network, and the improvements proposed such as road widening and intersection treatments, to cater for and mitigate the impact of project related traffic,
- Proposed road facilities, access and intersection treatments are to be identified and be in accordance with Austroads Guide to Road Design including provision of Safe Intersection Sight Distance (SISD),
- Local climate conditions that may affect road safety during the life of the project (e.g. fog, wet and dry weather, icy road conditions),

- Impact on public transport (public and school bus routes),
- Propose a Traffic Management Plan (TMP) to be developed following approval of the EIS, in consultation with relevant Councils and TfNSW. The TMP would need to identify strategies to manage the impacts of project related traffic, including any community consultation measures for peak haulage periods, and
- Propose a Driver Code of Conduct for haulage operations which could include, but not be limited to:
 - Safety initiatives for haulage through residential areas and/or school zones.
 - An induction process for vehicle operators and regular toolbox meetings.
 - A public complaint resolution and disciplinary procedure.

4 Existing Site Conditions Condition

Site Location

The location of the site is shown as Figure 1 and 2 below

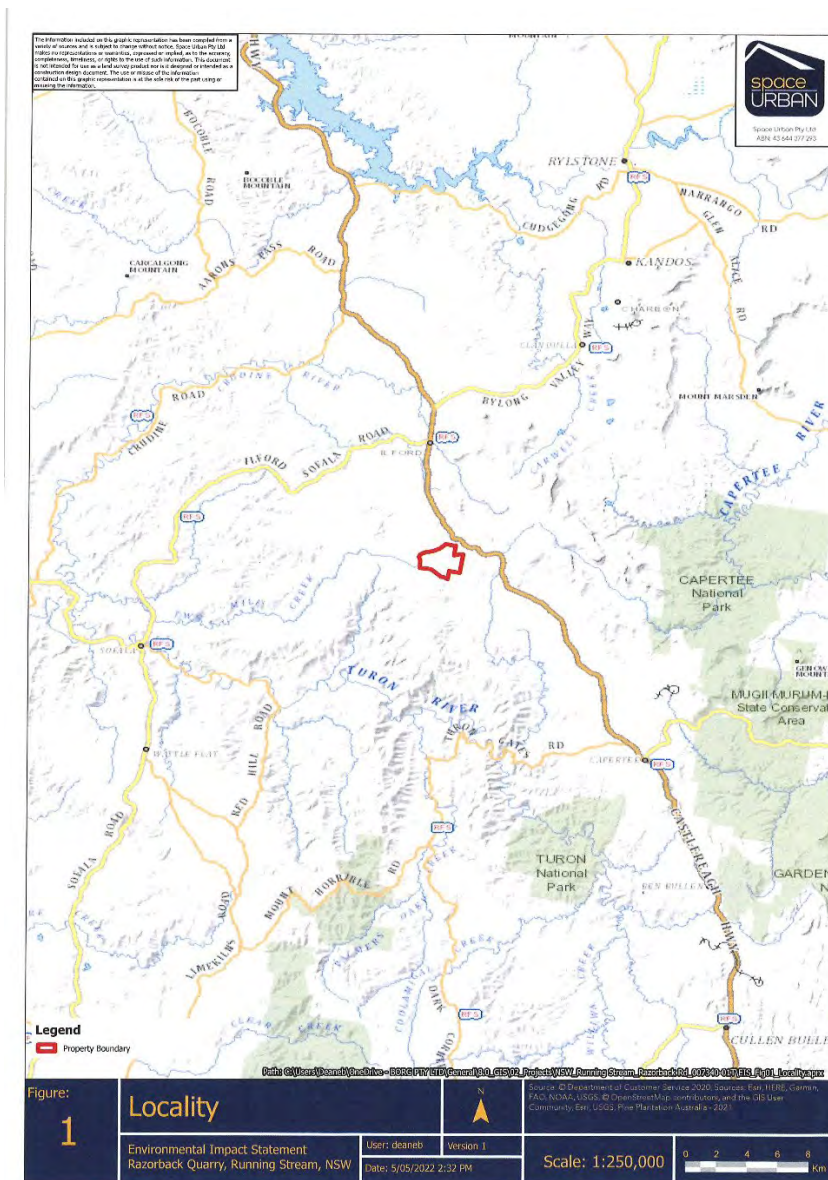


Figure 1 Site Location



Figure 2 Site location

Existing Approved Development

The majority of the subject land is used for pine plantations.

Surrounding lands are primarily larger agricultural holdings practising mixed grazing, along with a scattering of pine plantations and other uses.

Site Access

Access to the proposed quarry is off Razorback Road.

Razorback Road joins the Castlereagh Highway at a “T junction” which has been upgrade in the past to a standard CHR and BAL. Deceleration lengths and storage areas are consistent with current standards and traffic volumes.

The intersection with Razorback Road and the Castlereagh Highway consists of a left-turn deceleration lane for northbound vehicles on the highway and a protected right-turn for southbound vehicles turning into Razorback Road. There is no dedicated acceleration lane for vehicles turning south onto the Castlereagh Highway, however, an overtaking lane continues for south bound traffic for approximately 150m south of the intersection

Castlereagh Highway is a 100 km/h highway connecting Lithgow in the south with Mudgee in the north.

Razorback Road is a local road connecting the Castlereagh Highway with Turon Road, but primarily services local properties. The road is sealed for approximately 20m from the intersection with the Castlereagh Highway, beyond the sealed section the road is of gravel construction approximately 6 m in width.

Access into the proposed quarry site will be via an upgrade to an existing gravel entry road within private property.



Photograph 1: Looking west on Razorback Road toward the site entrance.



Photograph 2 - Looking east on Razorback Road at the intersection with the site entrance.



Photograph 3 - Intersection of Razorback Road and Castlereagh Highway.

5 Proposed Development

Proposed Development

It is proposed to develop and operate a sand and gravel quarry on the ‘Turonfels’ property at 39 Razorback Road, Running Stream as shown in Figure 3 (below).

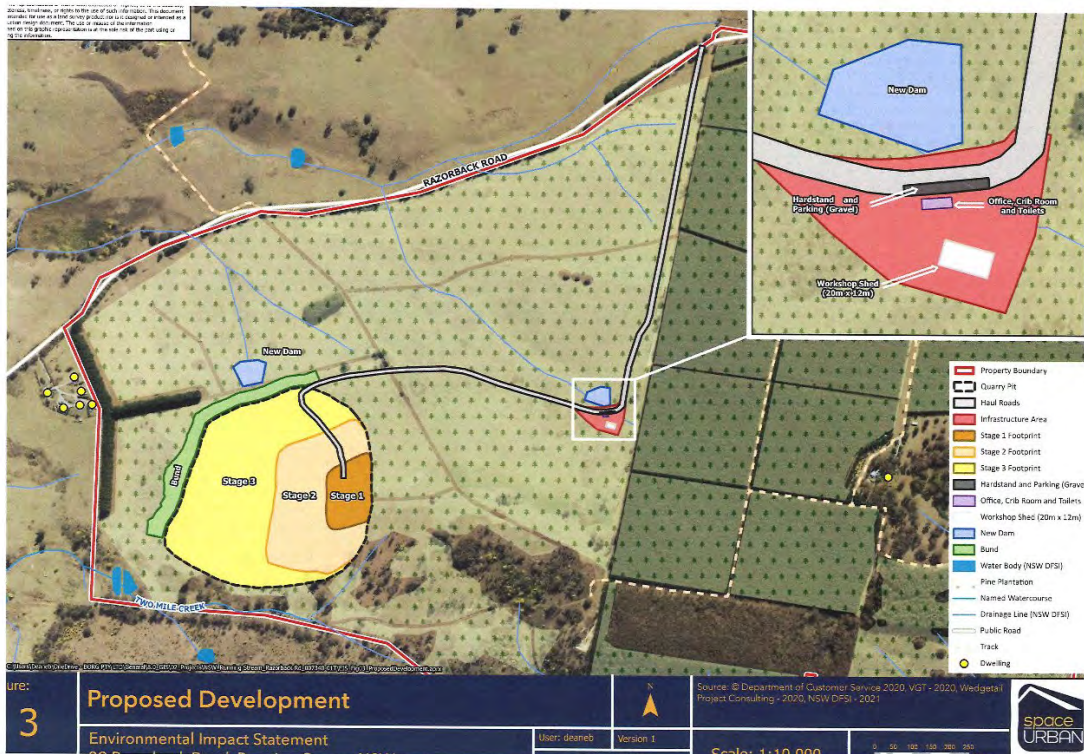


Figure 3 Proposed Site Layout

The proposed quarry will produce a maximum of 200,000 tonnes per annum (200 ktpa) of gravel and sand products, with two full time equivalent staff and a total site area of 24.7 ha including workshop, office and weighbridge facilities.

It is noted that the quarry material is loosely consolidated and will be extracted and processed without use of explosives, rock breakers or onsite crushers.

The quarry will be progressively rehabilitated to pasture and pine plantation with potential future use of the facilities area for forestry related activities.

Transport of products by road is proposed to be limited to 1,500 tonnes per day, or up to 5 truckloads per hour (10 movements in and out combined).

Hours of Operation

It is likely that the quarry will initially operate on a campaign basis to meet specific demands moving toward a potential full-time quarry pending product demand.

When operating, it is intended to operate the quarry under the following hours:

- Extraction and haulage:
 - Monday to Friday 7 am to 6 pm.
 - Saturday 8 am to 1 pm.
- No extraction or haulage activities on Sundays or public holidays.
- Incidental maintenance activities may occur outside the above times, but only where activities can be conducted and not be audible at neighbouring dwellings.

6 Traffic Generation

Trip Generation

Construction

All movements of people and materials will be via the road network.

Construction will occur over an estimated 12-week period during will include the following works:

- Bitumen sealing of Razorback Road to western property boundary,
- Construction of private haul road,
- Construction of workshop and crib hut,
- Construction of the weigh bridge, and
- Initial topsoil stripping and placement and planting of topsoil stockpiles as a noise bund along the western boundary of the quarry.

Quarry operations will commence once the above actions are completed.

Traffic movements during construction are anticipated to be:

- Light vehicle movements for construction workers 6 (i.e. 3 movements into and out of the site each day) movements generally in the morning and afternoon as construction workers arrive and leave the site,
- Delivery of construction materials for the site office, workshop, and weighbridge.
- Delivery of temporary construction worker toilets, lunchrooms, and site office,
- Mobilisation and de-mobilisation of heavy plant and equipment, and
- Delivery of concrete where required.

It is anticipated that heavy vehicles peak at 4 (i.e. 2 movements into and out of the site each day) vehicles arriving at the site to unload components).

Operations:

The proposed quarry operations are assumed to generate:

- Up to 5 laden trucks per hour exiting the Razorback Road intersection during operating hours (7:00 am to 6 pm Monday to Saturday).
- Up to 5 unladen trucks per hour entering the Razorback Road intersection during operating hours (7:00 am to 6 pm Monday to Saturday).

- Up to 4 vehicles of employees entering Razorback Road from approximately 7:00 am
- Up to 4 vehicles of employees leaving Razorback Road from approximately 5:00 pm

Decommissioning

At the end of quarries operational life of 30 years the development area will be decommissioned. During decommissioning, all above ground infrastructure would be removed and the land rehabilitated with vegetation consisting of pasture grasses initially to improve soil stability and then planted with pine consistent the adjacent pine plantation.

It is anticipated that similar vehicle movements as is in operational phase will continue in this decommissioning phase.

Traffic Distribution

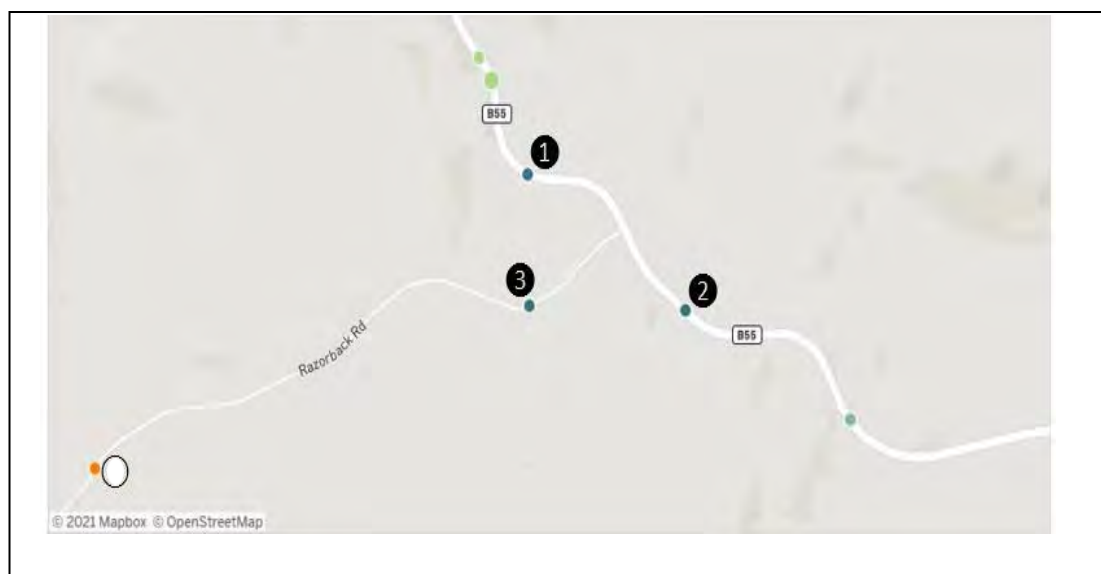
It is anticipated that for all phases of this development the distribution of traffic will be as follows:

- 50 / 50 split to the north and south for heavy vehicles, and
- 100% from the north for employees.

7 Traffic Impact at Intersections

Crash Data

Crash data from the NSW Centre for Road Safety Crash and Casualty Statistics Maps (2015-2019) shows no crashes at the intersection between 2015 and 2019. Two serious crashes recorded on the Castlereagh Highway and 1 serious crash and 1 injury/fatality crash recorded on Razorback Road are at a distance from the intersection.



	Crash ID	Degree of crash	RUM Code & Description	Road type	Lighting	Killed	Injured
①	1129642	Serious injury	80: Off left/right bend	2-way undivided	Dusk	-	3
②	1181491	Serious injury	74: On road, out of control	2-way undivided	Daylight	-	1
③	1183615	Serious injury	86: Off left/left bend	2-way undivided	Daylight	-	1
④	1092479	Fatal	20: Head on	2-way undivided	Daylight	1	6

Figure 4: NSW Centre for road Safety Crash and Casualty Statistics Map 2015 - 2019

Effect on Intersection Performance

Establishment of current traffic volumes

A traffic count was carried out on the 9 November 2021 between the hours of 8:00 am and 4:00 pm am to determine all traffic movements at the intersection. Results of the count are indicated below in Table 1.

Table 1 Criteria for Evaluating Capacity of Intersection

Date		9/11/2021															
Day of																	
Week		Tuesday															
		8:00		9:00		10:00		11:00		12:00		13:00		14:00		15:00	
Path		0 to 30	31 to 59	0 to 30	31 to 59	0 to 30	31 to 59	0 to 30	31 to 59	0 to 30	31 to 59	0 to 30	31 to 59	0 to 30	31 to 59	0 to 30	31 to 59
1A - LV		20	22	17	19	30	36	29	21	30	20	26	29	22	21	25	24
1A - HV		6	4	6	4	6	8	2	8	5	9	4	4	6	4	6	5
2 - LV		0	0	0	0	0	2	1	0	1	0	4	2	0	0	1	1
2 - HV		0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0
3 - LV		1	1	1	1	1	0	2	2	1	2	3	0	1	1	2	2
3 - HV		1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
4 - LV		0	1	0	1	0	0	2	1	1	0	2	1	1	2	1	1
4 - HV		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5 - LV		0	1	0	1	0	0	0	0	0	0	0	0	0	1	0	0
5 - HV		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6 - LV		17	18	17	18	21	27	25	24	24	20	25	21	16	20	26	25
6 - HV		6	5	6	5	6	3	6	2	5	3	0	3	2	7	3	3

In summary the following Peak hour movements were determined as shown table 2 below

Table 2 Maximum hourly traffic movements

	10 to 11 am	11 to 12 am
Path	140	143
1A - LV	66	65
1A - HV	14	10
2 - LV	2	3
2 - HV	0	0
3 - LV	1	2
3 - HV	0	0
4 - LV	0	2
4 - HV	0	0
5 - LV	0	0
5 - HV	0	0
6 - LV	48	52
6 - HV	9	9

Intersection Operation

How adequate the capacity of an intersection is judged by whether it can physically and operationally cater for the traffic using it.

The performances of the intersections relevant to the proposal have been assessed using the intersection modelling SIDRA software. The model provides parameters of the performance of an intersection including the degree of saturation (DoS) and the average delay per vehicle. It provides an accurate and consistent guide to the performance of an intersection under the different traffic flow scenarios. The recommended criteria for evaluating capacity of intersections are shown in Table 3

Table 3 Criteria for Evaluating Capacity of Intersection

Level of Service	Degree of Saturation (DoS)	Ave. Delay/ Veh. (Secs)
A/B good operation	less than 0.80	Less than 28
C satisfactory	0.80 to 0.85	29-42
D poor but manageable	0.85 to 0.90	43-56
E at capacity	0.90 to 1.0	57-70
F unsatisfactory, extra capacity required	Over 1.0	Over 70

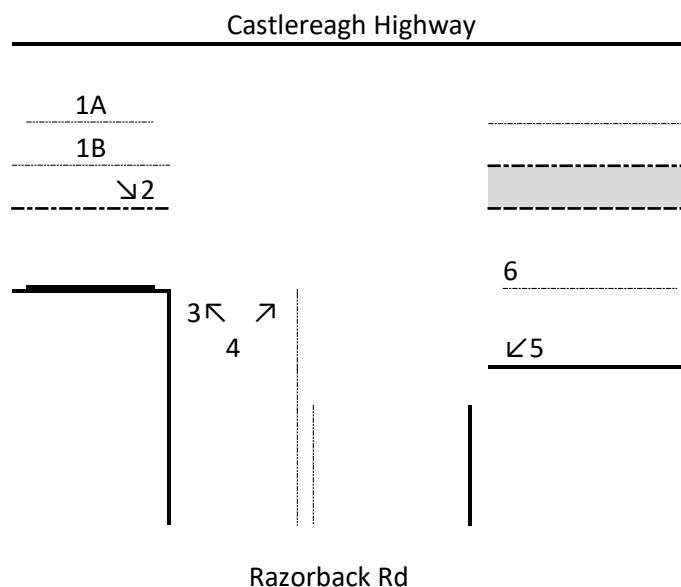
In the absence of historical growth figures, a conservative 3% traffic growth rate was applied to the 2021 traffic counts to determine a 10-year forecast.

To determine if the proposed movements would have an effect on the operations of the existing intersection a SIDRA analysis was carried out.

The analysis has been carried out for the following scenarios:

- Existing 2021,
- Existing 2021 with proposed development,
- 10-year planning horizon (with above assumed growth rate), and
- 10-year planning horizon with proposed development.

Table 4 Criteria for Evaluating Capacity of Intersection



Path	Existing Movements	Post development	Existing plus post	10 years growth	Post Development 10 years growth
1A - LV	66	0	66	86	86
1A - HV	14	0	14	18	18
2 - LV	2	4	6	3	7
2 - HV	0	3	3	0	3
3 - LV	1	0	1	1	1
3 - HV	0	3	3	0	3
4 - LV	0	0	0	0	0
4 - HV	0	2	2	0	2
5 - LV	0	0	0	0	0
5 - HV	0	2	2	0	2
6 - LV	48	0	48	62	62
6 - HV	9	0	9	12	12

Traffic Modelling Assumption

- Analysis was carried out for the maximum hour flow as shown in Table 2 only as worst case scenario for traffic,
- Existing intersection geometry, including lane lengths and widths were measured using engineering survey,
- SIDRA default values were adopted,
- Level of Services Method is set to RTA NSW,
- Speed environment 100 km/hr on Castlereagh Highway and 50 km/hr on Razorback Rd,
- Length of right turn 2 is 90 plus deacceleration lane,
- Length of left turn 5 is 50 m plus deacceleration lane, and
- Heavy Vehicles (HV) 90% in lane 1A and 10% in lane 1B

Full details of the outputs are found in Appendix B. however a summary of this analysis is provided below

Table 5 Intersection Performance (AM Peak) South Bound Castlereagh Highway

Criteria	Base	Base with Development	Base with 10 years growth	10 Years with Development
Av. Delay (sec)	0.0	0.0	0.0	0.0
Level of Service	A	A	A	A

Table 6 Intersection Performance (AM Peak) North Bound Castlereagh Highway.

Criteria	Base	Base with Development	Base with 10 years growth	10 Years with Development
Av. Delay (sec)	0.0	0.0	0.0	0.0
Level of Service	A	A	A	A

Table 7 Intersection Performance (AM Peak Right Turn into Razorback Rd)

Criteria	Base	Base with Development	Base with 10 years growth	10 Years with Development
Av. Delay (sec)	7.7	8.7	7.8	8.7
Level of Service	A	A	A	A
Q. Length- (m)	0	0	0	0

Table 8 Intersection Performance (AM peak Right turn out of Razorback Rd)

Criteria	Base	Base with Development	Base with 10 years growth	10 Years with Development
Av. Delay (sec)	8.6	14.4	9	15.6
Level of Service	A	A	A	A
Q. Length- (m)	0	0	0	0

Table 9 Intersection Performance (AM peak Left turn out of Razorback Rd)

Criteria	Base	Base with Development	Base with 10 years growth	10 Years with Development
Av. Delay (sec)	7.7	10.8	9	11
Level of Service	A	A	A	A
Q. Length- (m)	0	0	0	0

Table 10 Intersection Performance (AM peak Left turn into Razorback Road)

Criteria	Base	Base with Development	Base with 10 years growth	10 Years with Development
Av. Delay (sec)	7.8	10.4	7.8	9.6

Level of Service	A	A	A	A
Q. Length- (m)	0	0	0	0

The modelling outputs as shown in Table 5 through 10 illustrate that there is no deterioration of Av Delay, Level of Service, or Que length when development traffic is added to either of the 2021 or 2031 simulations of the intersection.

In 2031 (in either scenario) the intersection operates at Level of Service of A on all legs and turn movements in the morning peak hour.

Further the no que length for the right turn into Razorback Rd under the development scenario in 10 years is evident indicating that the 90 m of available for storage of the current road layout is more than adequate.

Sight Distance and Visibility Issues

Austrroads guidelines provide general parameter values, which they refer to as the Normal Design Domain (NDD).

This report discusses the existing intersection in terms of Normal Design Domain criteria only

As illustrated in Figure 4, Safe Intersection Sight Distance (SISD) is measured from a driver eye height of 1.1 m above the road to a point 1.25 m above the road, which represents the upper part of a car. It is measured along the carriageway from the approaching vehicle to a conflict point.

Austrroads SISD allows the use of a 1.5 seconds’ or 2.0 seconds observation time for T-intersections on single carriageway roads that have a traffic volume less than 4,000 vehicles per day with the minor road having a traffic volume of less than 400 vehicles per day.

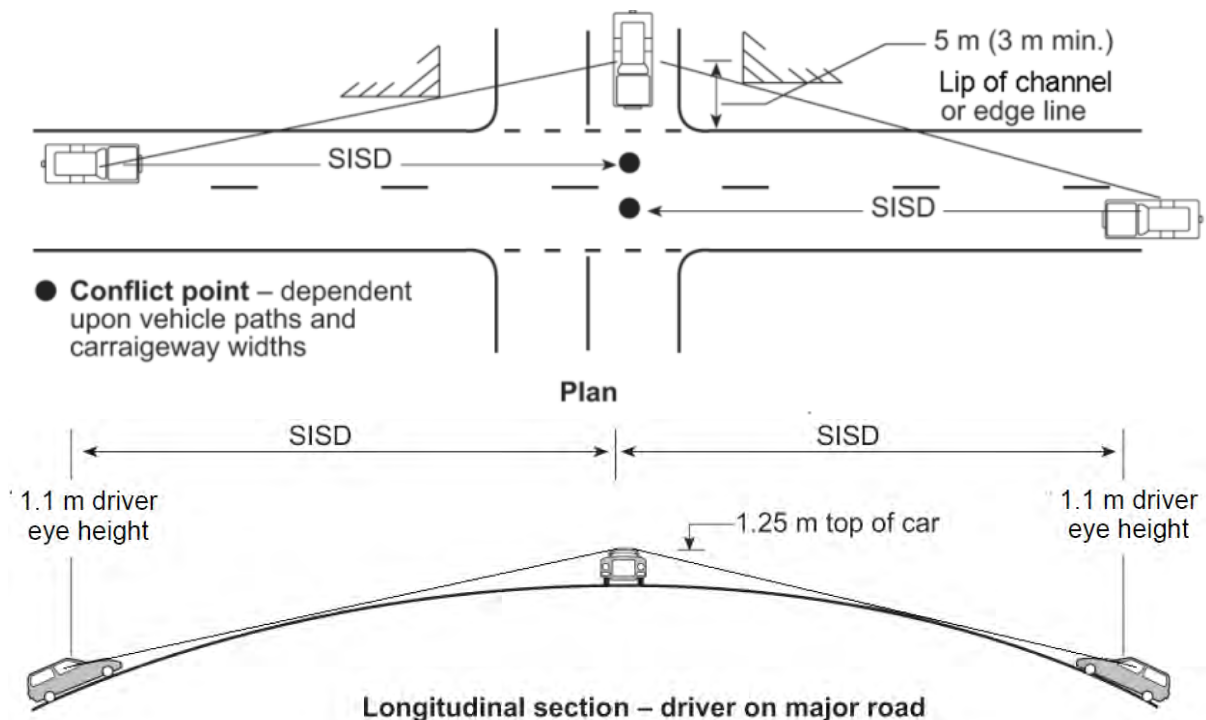


Figure 5: SISD diagrams from Austrroads, Figure 3.2

Table 11 Intersection Performance (AM peak Left turn into Razorback Road)

Safe Intersection Sight Distance values are: Design speed (km/h)	Reaction time (seconds)	SISD (m)	Correction for 8% grade (m)	Resultant SISD (m)
100	1.5	234	-20	214
100	2.0	248	-20	228

Exiting Safe Intersection Sight Distance to the South

Available SISD to and from the south of Razorback Road was measured at greater 250 m from engineering survey and as such meets the Austroads guidelines for both reaction time of 1.5 and 2 seconds. Full details including long sections are shown on Arkhill Engineers drawing on SK4153-012 sight line 2 in Appendix B.



Photograph 4: Looking south from Razorback Road to an approaching vehicle

Existing Intersection Safe Sight Distance to the north.

Available SISD to and from the north from Razorback Road was measured at 135 m to 140 m, from engineering survey. This does not meet the requirements in Austroads for this speed environment.

As shown in Photograph 5 and 6, sight distance north to the highway is restricted by a vegetated embankment on the northern side of Razorback Road.

Signage and vegetation at the toe of the embankment batter further obscure the sight line.



Photograph 5 and 6: Looking north from Razorback Road to an approaching vehicle

Due to the reduced site distance and the impact of vegetation and embankment to the north it is proposed to cut back this embankment to achieve the desired site distances detailed in table 11. The concept for these proposed earthworks is discussed in section 9 below.

8 Impact on public transport

There is no impact on public transport as traffic movements are minimal.

9 Proposed Road Works and Intersection Works

To address the increase traffic movements, highlighted above it is proposed to carry out the following works:

Bitumen sealing of Razorback Road

The applicant proposes to bitumen seal, to Council requirements, that section of Razorback Road from the existing seal near Castlereagh Highway to 15m west of the entrance to the quarry. Such sealing will ensure that the minor increase in vehicle movements will not have an adverse effect on road safety or the amenity of adjacent properties.

Improved intersection warning signage.

The current Castlereagh Highway “side road intersection” sign on approach from the north is installed alongside CAMs and may not be obvious to drivers, reducing driver awareness of the Razorback Road intersection.

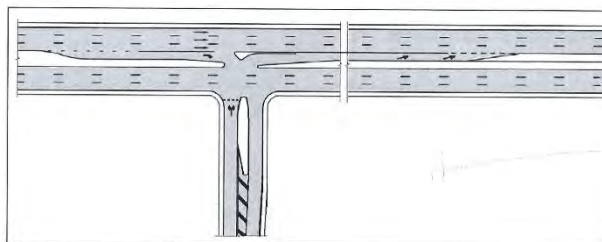
Installing a larger sized sign, repositioned to be clear of the CAMs and supplemented with a *TRUCKS CROSSING OR ENTERING* sign would reinforce to drivers that they are approaching an intersection where heavy vehicles may be entering the highway, assisting with driver awareness.

Improvement of sight distance to the north.

In light of the comments provide in Section 7 two alternatives were examined to determine if and how an improved sight distance could be achieve, namely provision of an acceleration lane to the south or improving sight distance to the north by removing some of the embankment and vegetation.

Acceleration Lane

Austrroads and TfNSW standards sets out criteria for minimum length of acceleration lanes on the departure side of intersections. Section 5.5 Austrroads Guide to Road Design Part 4A 2010 - Intersections provides details of acceptable design lengths for heavy vehicle acceleration lanes as shown in figure 6 below.



Notes:
 1. Schematic sketch only. Refer to Figure 7.14 for design details.
 2. Arrows indicate movements relevant to the turn type. They do not represent actual pavement markings.
 Figure 4.19: Seagull treatment (preferred)

Figure 6 – Acceleration lane on Departure side of seagull Intesection treatment

This guide (Austroads) provides a range of lengths for acceleration lanes for different road environments and speeds based on a heavy vehicle achieving a speed 20km/hr below the mean free speed of the through traffic when it merges.

Based on current TfNSW comments the mean free speed would be the speed limit at this location (ie 100km/h.) Hence a heavy vehicle will need to accelerate to 80 km/hr prior to merging.

In this location the down hill grade of up to 4 % will assist in reducing the length but in the absence of accurate survey data an acceleration lane in the order of 400m to 500m may be required for a heavy vehicle to come up to speed.

Table 5.7: Acceleration lane lengths (m) for semi-trailers to accelerate from rest to a specified speed on a level or downgrade

Downgrade (%)	Truck speed (km/h)				
	100	90	80	70	60
0	2400	1500	910	550	320
1	1400	940	540	410	250
2	970	700	500	330	210
3	760	560	400	260	180

Figure 7 Extract form Austroads

Based on the above table and free speed of 100km/hr, figure 8 below indicates the possible extent of an acceleration lane meeting the criteria set out above.



Figure 8 Extent of Accelaraion lane

A site inspection identified the following issues that will need to be accommodated in the development of a concept design of the proposed improvement.

- Interaction of the merge of the two traveling lanes into one at the same location as the merge of the acceleration lane into the through lane.
- The road geometry (curvature) has the risk of reducing sight lines due to vegetation on the inside of the curve (including in private property) that might need to be removed.
- To meet the TfNSW requirement the existing CHS may need to be converted into a seagull intersection to ensure that vehicles entering the acceleration lane are protected from through traffic, and
- As shown in photographs 7 and 8 below there appears to be a steep drop off from the existing pavement and it may be difficult to contain the road batter with the additional pavement widening required within the road reserve.



Photograph 7 Embankment inside of curve southbound Castlereagh Highway



Photograph 8 Embankment inside of curve southbound Castlereagh Highway

Improving site lines to the north

As mentioned above, sight distance north to the highway is restricted by a vegetated embankment on the northern side of Razorback Road. Signage and vegetation at the toe of the embankment batter further obscure the sight line.

An alternative option to avoid a long acceleration lane would be to trim the embankment on the northern side of the intersection to improve the site lines

Figures 7 and 8 below indicates that clearin and excavation od embankment to achieve the desired sire distance of 205 m.

Discussion are underway to purchase the land required for this activity/



Figure 9 Extent of embankment works



Figure 10 Extent of embankment works

As shown in photographs 9, 10 and 11 the embankment appears to be suitable to bench back and stand at a steep slope without the need for a retaining wall. A slope stability investigation would need to be carried out to determine the appropriate batter slope.

An earth catch drain at the top of the batter and a rock lined dish drain at the bottom of the batter has been provided to protect the batter and road pavement from surface water shedding from the area above the batter.



Photograph 9 Embankment to north of Intesection



Photograph 10 Embankment to north of Intesection



Photograph 11 Embankment to north of Intesection

No other pavement widening works would be required for this option.

As indicated in Section 9 it is clear that the most engineering feasible solution would be to improve sight distance to the north and it is proposed to layback the existing embankment and remove vegetation to ensure that the required Austroads sight distance is achieved.

Arkhill Engineers have completed a concept design (see Appendix B) which proves details of the extent of work required and the final landform.

Arkhill Engineers Concept Design detailed in Appendix B provides the following details:

- SK 4153-001 Site plan showing sight lines and proposed earthworks,
- SK 41530-002 Proposed upgrading to line marking,
- SK 4153-005 details of proposed earthworks,
- SK 4153-001 existing long section along centre line of Castlereagh Highway to the north,
- SK4153-012 detailing long-sections along each of the 4 nominated sight lines showing clearance above existing pavement for sight lines 1 to 3 and the obstruction caused by the embankment to sight line 4,
- SK1553-021 to 023 Details of existing sections through the embankment, and
- SK4153-015 and SK4153-025 to 027 details the proposed excavation required to achieve a clear sightline 4.

This design clearly demonstrates that such an excise of modifying the existing embankment will ensure satisfactory sight lines are achieved to the north in a cost-effective method. It should be noted that the applicant is in discussions to acquire the land required to be excavated. Materials from the excavation will be used in the development of the quarry, haul roads and noise bunds.

10 Traffic Management Plan and Driver Code of Conduct

A draft Operational Traffic Management Plan (TMP) is provided in Appendix A, together with a Draft Driver Code of Conduct. This will be finalised upon approval of the development and after consultation with Mid Western Council and TfNSW.

11 Summary

The traffic impacts from the combined development have been assessed and the key findings are as follows:

- The available sight distance of Razorback Road to the south along Castlereagh Highway is adequate for the speed environment.
- The available sight distance of Razorback Road to the north along Castlereagh Highway is inadequate for the speed environment. However, a proposed concept design has been developed to trim back the embank to the north and this design provides a clear sight distance meeting Austroads guidelines.
- Total traffic generation remains low and has no impact on the intersection performance and demonstrates that the current protected right turn storage and left turn de acceleration lane is adequate and no other intersection improvements are necessary.
- Minor signage upgrades are warranted to improve the awareness of the approaching intersections.
- Sealing of Razorback Road to 15m west of the quarry access will ensure that the minor increase in vehicle movements will not have an adverse effect on road safety or amenity of adjacent properties.

Based on the findings of this report, Pavey Consulting Services is of the opinion that there are no traffic engineering related matters that should preclude approval of this Development Application.

Prepared by:
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B.E (Civil) Grad Dip LGE. LGE Cert MAICD, MAIPM
Director,
Pavey Consulting Services

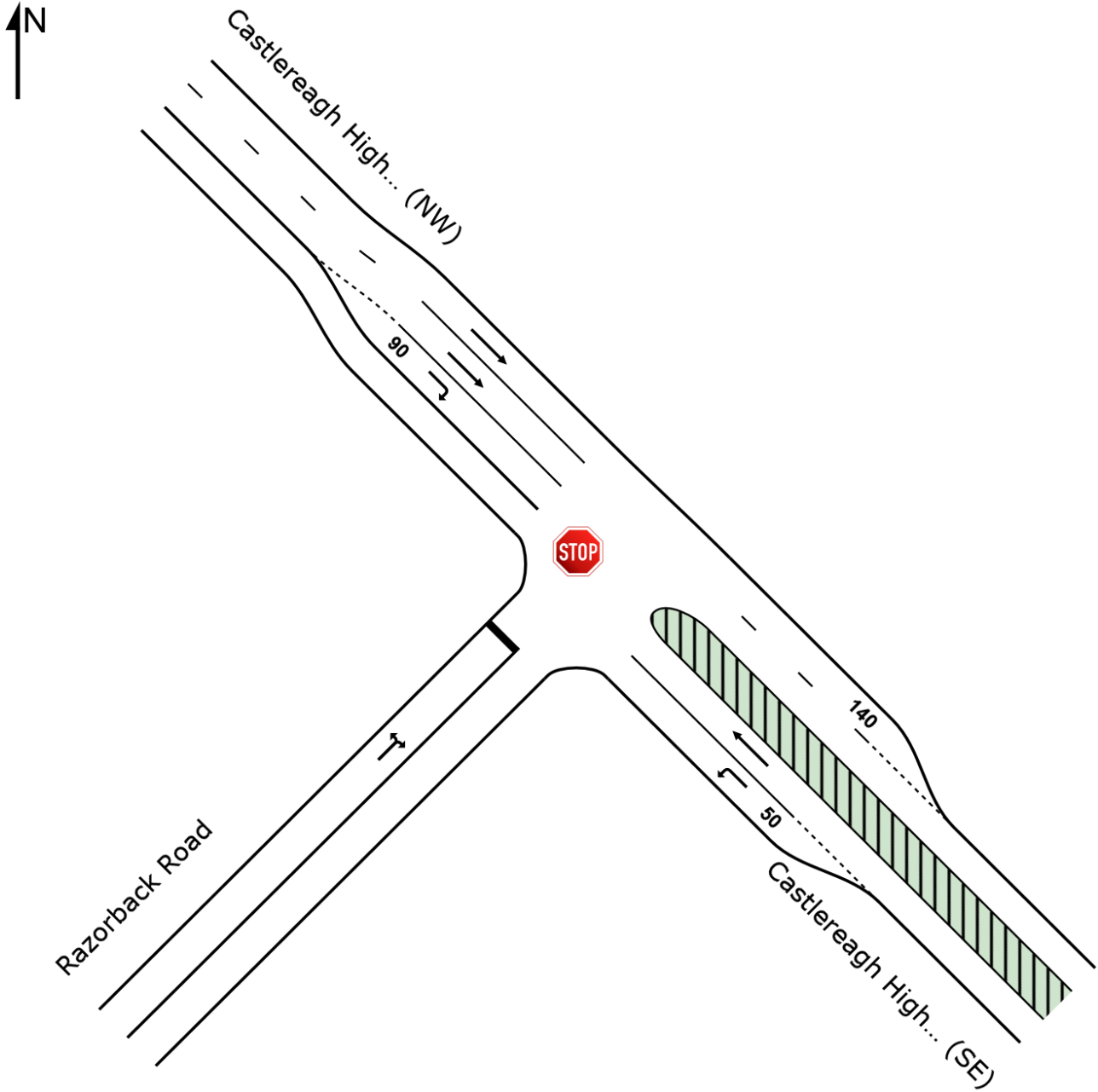
Appendix A – SIDRA OUTPUTS

SITE LAYOUT

 Site: [2021_CastlereaghHwy-RazorbackRd_BY_PeakHour_v01 (Site Folder: General)]

Castlereagh Highway and Razorback Road Intersection
Base Year (2021, existing)
Peak Hour
Site Category: (None)
Stop (Two-Way)

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



MOVEMENT SUMMARY

 Site: [2021_CastlereaghHwy-RazorbackRd_BY_PeakHour_v01 (Site Folder: General)]

Castlereagh Highway and Razorback Road Intersection
 Base Year (2021, existing)
 Peak Hour
 Site Category: (None)
 Stop (Two-Way)

Vehicle Movement Performance														
Mov ID	Turn	INPUT VOLUMES		DEMAND FLOWS		Deg. Satn	Aver. Delay	Level of Service	95% BACK OF QUEUE		Prop. Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed
		[Total veh/h]	[HV] veh/h	[Total veh/h]	[HV] %				[Veh. veh]	[Dist] m				
SouthEast: Castlereagh Highway (SE)														
21	L2	1	0	1	0.0	0.001	7.8	LOS A	0.0	0.0	0.00	0.66	0.00	75.3
22	T1	57	9	60	15.8	0.034	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
Approach		58	9	61	15.5	0.034	0.1	NA	0.0	0.0	0.00	0.01	0.00	99.4
NorthWest: Castlereagh Highway (NW)														
28	T1	80	14	84	17.5	0.028	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
29	R2	2	0	2	0.0	0.002	7.7	LOS A	0.0	0.0	0.15	0.61	0.15	57.1
Approach		82	14	86	17.1	0.028	0.2	NA	0.0	0.0	0.00	0.01	0.00	98.2
SouthWest: Razorback Road														
30	L2	1	0	1	0.0	0.002	7.7	LOS A	0.0	0.1	0.19	0.88	0.19	54.6
32	R2	1	0	1	0.0	0.002	8.6	LOS A	0.0	0.1	0.19	0.88	0.19	54.5
Approach		2	0	2	0.0	0.002	8.1	LOS A	0.0	0.1	0.19	0.88	0.19	54.5
All Vehicles		142	23	149	16.2	0.034	0.3	NA	0.0	0.1	0.00	0.03	0.00	97.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
 Vehicle movement LOS values are based on average delay per movement.
 Minor Road Approach LOS values are based on average delay for all vehicle movements.
 NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
 Delay Model: SIDRA Standard (Geometric Delay is included).
 Queue Model: SIDRA Standard.
 Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
 HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

MOVEMENT SUMMARY

 Site: [2021_CastlereaghHwy-RazorbackRd_BC_PeakHour_v01 (Site Folder: General)]

Castlereagh Highway and Razorback Road Intersection
 Base Case (existing + development)
 Peak Hour
 Site Category: (None)
 Stop (Two-Way)

Vehicle Movement Performance														
Mov ID	Turn	INPUT VOLUMES		DEMAND FLOWS		Deg. Satn	Aver. Delay	Level of Service	95% BACK OF QUEUE		Prop. Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed
		[Total veh/h]	[HV] veh/h	[Total veh/h]	[HV] %				[Veh. veh]	[Dist] m				
SouthEast: Castlereagh Highway (SE)														
21	L2	2	2	2	100.0	0.002	10.4	LOS A	0.0	0.0	0.00	0.67	0.00	52.3
22	T1	57	9	60	15.8	0.034	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
Approach		59	11	62	18.6	0.034	0.4	NA	0.0	0.0	0.00	0.02	0.00	97.0
NorthWest: Castlereagh Highway (NW)														
28	T1	80	14	84	17.5	0.028	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
29	R2	9	3	9	33.3	0.008	8.7	LOS A	0.0	0.3	0.17	0.62	0.17	56.2
Approach		89	17	94	19.1	0.028	0.9	NA	0.0	0.3	0.02	0.06	0.02	92.7
SouthWest: Razorback Road														
30	L2	4	3	4	75.0	0.010	10.8	LOS A	0.0	0.4	0.22	0.99	0.22	41.0
32	R2	2	2	2	100.0	0.010	14.4	LOS A	0.0	0.4	0.22	0.99	0.22	41.1
Approach		6	5	6	83.3	0.010	12.0	LOS A	0.0	0.4	0.22	0.99	0.22	41.0
All Vehicles		154	33	162	21.4	0.034	1.1	NA	0.0	0.4	0.02	0.08	0.02	89.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
 Vehicle movement LOS values are based on average delay per movement.
 Minor Road Approach LOS values are based on average delay for all vehicle movements.
 NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
 Delay Model: SIDRA Standard (Geometric Delay is included).
 Queue Model: SIDRA Standard.
 Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
 HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

MOVEMENT SUMMARY

 Site: [2021_CastlereaghHwy-RazorbackRd_FY_PeakHour_v01 (Site Folder: General)]

Castlereagh Highway and Razorback Road Intersection
 Future Year (10 years projection)
 Peak Hour
 Site Category: (None)
 Stop (Two-Way)

Vehicle Movement Performance														
Mov ID	Turn	INPUT VOLUMES		DEMAND FLOWS		Deg. Satn	Aver. Delay	Level of Service	95% BACK OF QUEUE		Prop. Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed
		[Total veh/h]	[HV] veh/h	[Total veh/h]	[HV] %				[Veh. veh]	[Dist] m				
SouthEast: Castlereagh Highway (SE)														
21	L2	1	0	1	0.0	0.001	7.8	LOS A	0.0	0.0	0.00	0.66	0.00	75.3
22	T1	74	12	78	16.2	0.044	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
Approach		75	12	79	16.0	0.044	0.1	NA	0.0	0.0	0.00	0.01	0.00	99.5
NorthWest: Castlereagh Highway (NW)														
28	T1	104	18	109	17.3	0.036	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
29	R2	3	0	3	0.0	0.002	7.8	LOS A	0.0	0.1	0.18	0.60	0.18	57.0
Approach		107	18	113	16.8	0.036	0.3	NA	0.0	0.1	0.00	0.02	0.00	97.9
SouthWest: Razorback Road														
30	L2	1	0	1	0.0	0.003	7.8	LOS A	0.0	0.1	0.23	0.86	0.23	54.4
32	R2	1	0	1	0.0	0.003	9.0	LOS A	0.0	0.1	0.23	0.86	0.23	54.3
Approach		2	0	2	0.0	0.003	8.4	LOS A	0.0	0.1	0.23	0.86	0.23	54.4
All Vehicles		184	30	194	16.3	0.044	0.3	NA	0.0	0.1	0.01	0.02	0.01	97.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
 Vehicle movement LOS values are based on average delay per movement.
 Minor Road Approach LOS values are based on average delay for all vehicle movements.
 NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
 Delay Model: SIDRA Standard (Geometric Delay is included).
 Queue Model: SIDRA Standard.
 Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
 HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

MOVEMENT SUMMARY

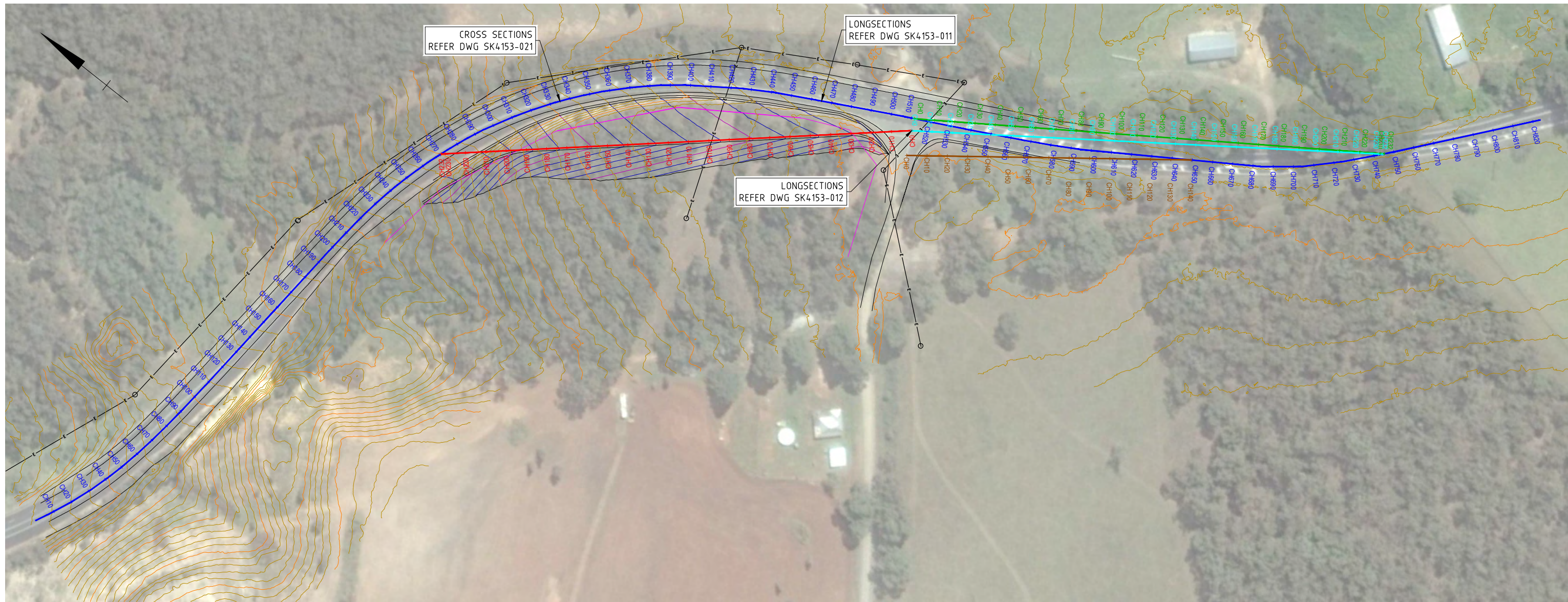
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Castlereagh Highway and Razorback Road Intersection
 Future Case (10 years projection with development)
 Peak Hour
 Site Category: (None)
 Stop (Two-Way)

Vehicle Movement Performance														
Mov ID	Turn	INPUT VOLUMES		DEMAND FLOWS		Deg. Satn	Aver. Delay	Level of Service	95% BACK OF QUEUE		Prop. Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed
		[Total veh/h]	[HV] veh/h	[Total veh/h]	[HV] %				[Veh. veh]	[Dist] m				
SouthEast: Castlereagh Highway (SE)														
21	L2	3	2	3	66.7	0.003	9.6	LOS A	0.0	0.0	0.00	0.66	0.00	55.6
22	T1	74	12	78	16.2	0.044	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
Approach		77	14	81	18.2	0.044	0.4	NA	0.0	0.0	0.00	0.03	0.00	97.0
NorthWest: Castlereagh Highway (NW)														
28	T1	104	18	109	17.3	0.036	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	100.0
29	R2	10	3	11	30.0	0.009	8.7	LOS A	0.0	0.3	0.20	0.61	0.20	56.2
Approach		114	21	120	18.4	0.036	0.8	NA	0.0	0.3	0.02	0.05	0.02	93.6
SouthWest: Razorback Road														
30	L2	4	3	4	75.0	0.011	11.0	LOS A	0.0	0.5	0.26	0.98	0.26	40.8
32	R2	2	2	2	100.0	0.011	15.6	LOS B	0.0	0.5	0.26	0.98	0.26	40.9
Approach		6	5	6	83.3	0.011	12.5	LOS A	0.0	0.5	0.26	0.98	0.26	40.9
All Vehicles		197	40	207	20.3	0.044	1.0	NA	0.0	0.5	0.02	0.07	0.02	91.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
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 Minor Road Approach LOS values are based on average delay for all vehicle movements.
 NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
 Delay Model: SIDRA Standard (Geometric Delay is included).
 Queue Model: SIDRA Standard.
 Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
 HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Appendix B – Arkhill Engineers Concept Design



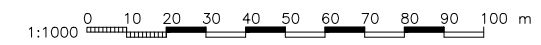
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REFER DWG SK4153-021

LONGSECTIONS
REFER DWG SK4153-011

LONGSECTIONS
REFER DWG SK4153-012

LEGEND:

- WEST SIGHT
- EAST SIGHT 1
- EAST SIGHT 2
- EAST SIGHT 3
- CASTLEREAGH HIGHWAY CENTRELINE
- POWERLINE
- PHIL ORR BDY LINE



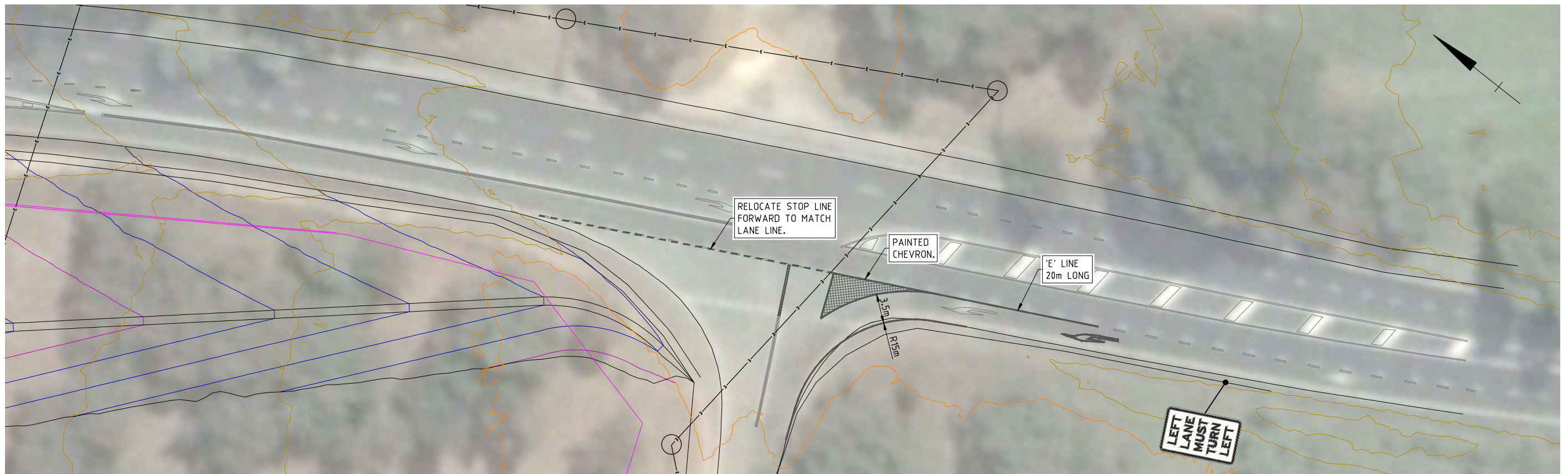
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DESIGNED: A.M.
DRAWN: A.M.
APPROVED: -



53 BONVILLE AVENUE THORNTON NSW 2322 PO BOX 29 MAITLAND NSW 2320		ABN: 54 003 867 374 Phone: (02) 4088 0700 FAX: (02) 4964 2104 Email: arkhill@arkhill.com.au	
TITLE BORG RUNNING CREEK - RAZORBACK RD INTERSECTION SIGHT DISTANCE - SITE PLAN			
SCALE	SIZE	SKETCH No.	REVISION
1:1,000	A1	SK4153-001	0
		DATE	13-01-22



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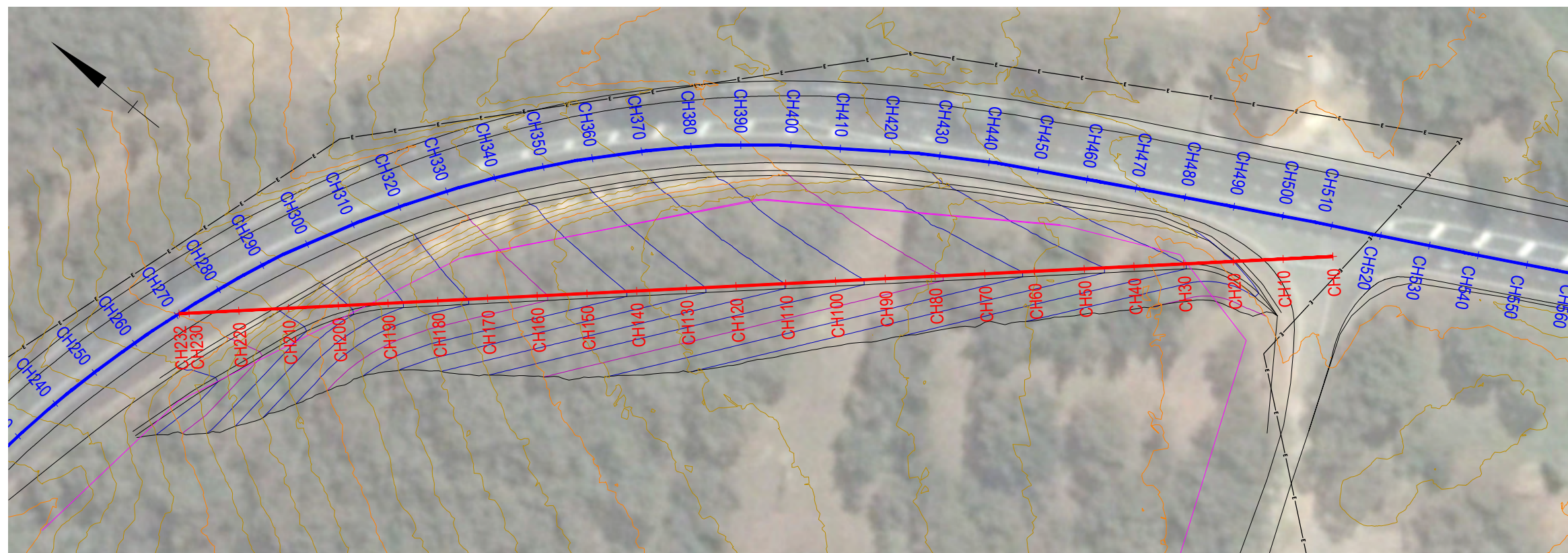


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MANTLAND NSW 2320

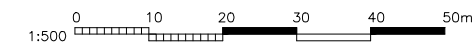
ABN: 54 003 867 374
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FAX: (02) 4964 2104
Email: arkhill@arkhill.com.au

DESIGNED:	A.M.
DRAWN:	B.S.
APPROVED:	-

TITLE			
BORG RUNNING CREEK - RAZORBACK RD INTERSECTION SIGHT DISTANCE - LINEMARKING PLAN			
SCALE	SIZE	SKETCH No.	REVISION DATE
1:250	A1	SK4153-002	0 18-01-22



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DESIGNED: A.M.
DRAWN: B.S.
APPROVED: -

TITLE
**BORG RUNNING CREEK - RAZORBACK RD INTERSECTION
EARTHWORKS WEST SIGHT LINE - SITE PLAN**

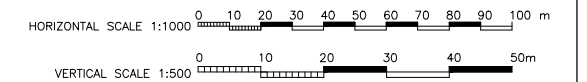
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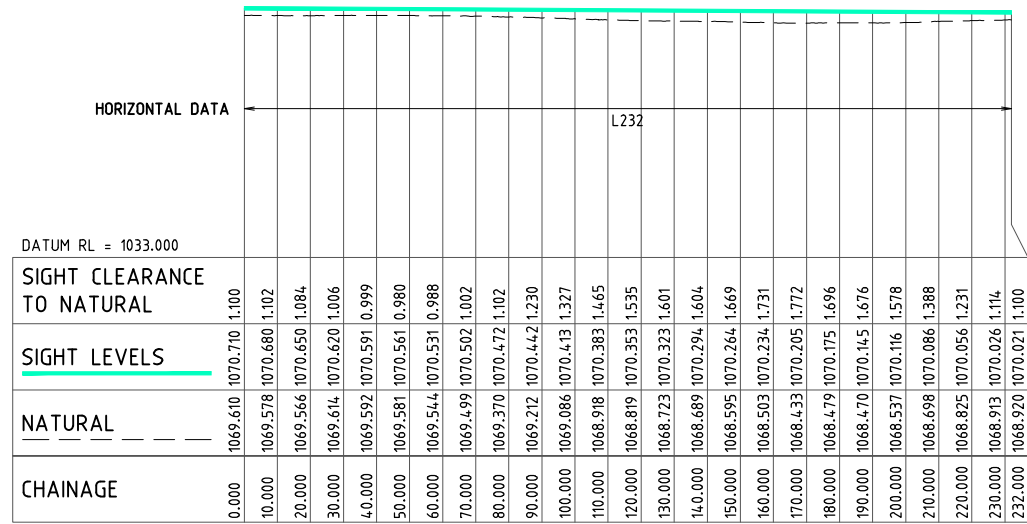
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CHAINAGE	0.000	10.000	20.000	30.000	40.000	50.000	60.000	70.000	80.000	90.000	100.000	110.000	120.000	130.000	140.000	150.000	160.000	170.000	180.000	190.000	200.000	210.000	220.000	230.000	240.000	250.000	260.000	270.000	280.000	290.000	300.000	310.000	320.000	330.000	340.000	350.000	360.000	370.000	380.000	390.000	400.000	410.000	420.000	430.000	440.000	450.000	460.000	470.000	480.000	490.000	500.000	510.000	520.000	530.000	540.000	550.000	560.000	570.000	580.000	590.000	600.000	610.000	620.000	630.000	640.000	650.000	660.000	670.000	680.000	690.000	700.000	710.000	720.000	730.000	740.000	750.000	760.000	770.000	780.000	790.000	800.000	810.000	820.000	823.804	

CASTLEREAGH HIGHWAY CENTRELINE - LONGSECTION

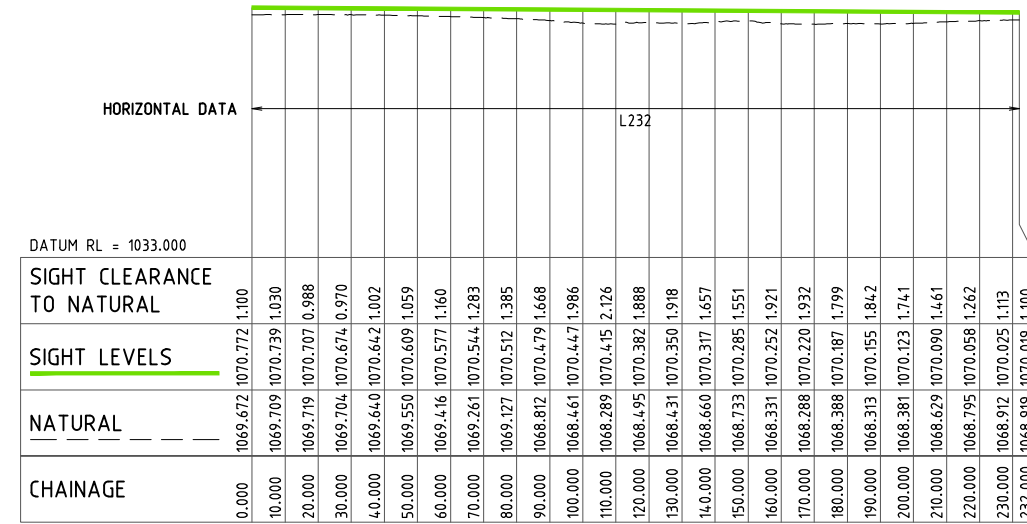


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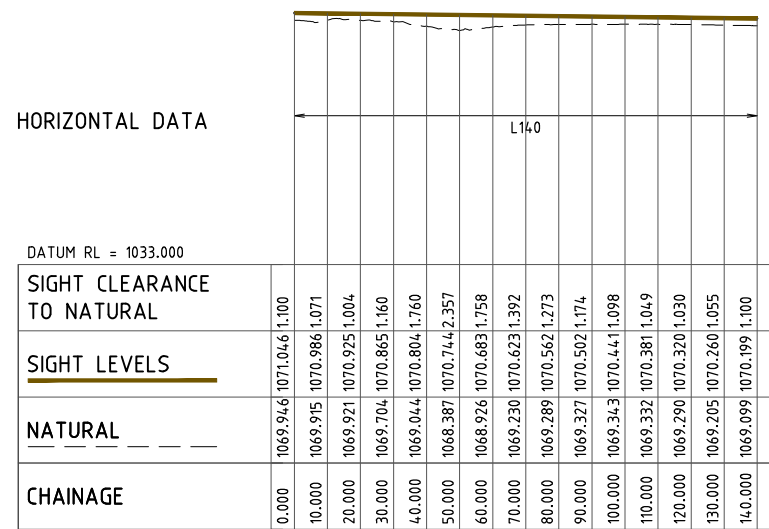
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DESIGNED:	A.M.	TITLE BORG RUNNING CREEK - RAZORBACK RD INTERSECTION LONGSECTION SHEET 1 OF 2			
DRAWN:	A.M.	SCALE	SIZE	SKETCH No.	REVISION DATE
APPROVED:		AS SHOWN	B1	SK4153-011	0 13-01-22



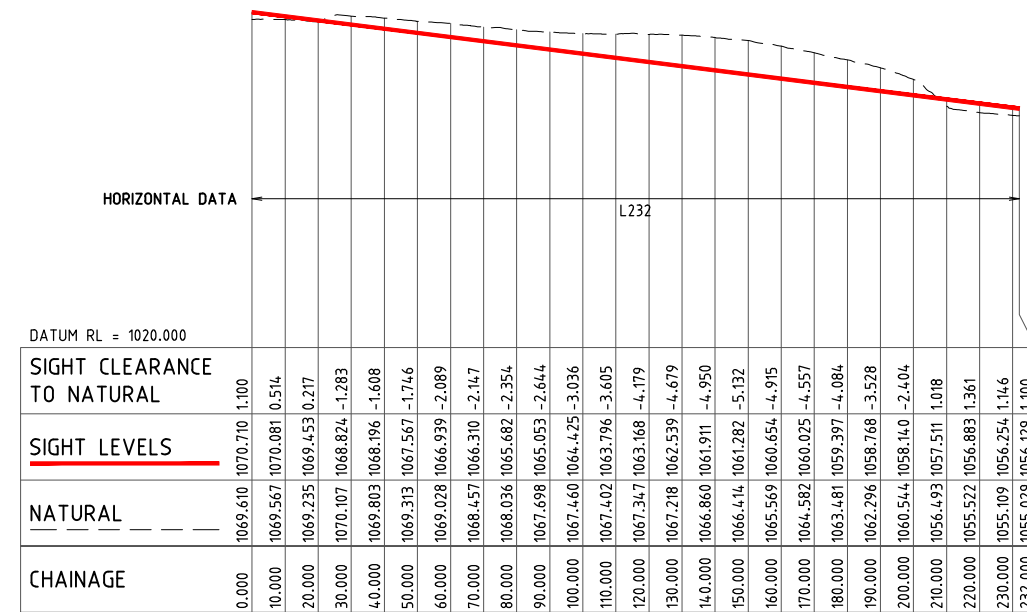
EAST SIGHT 1 - LONGSECTION



EAST SIGHT 2 - LONGSECTION

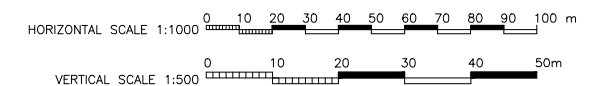


EAST SIGHT 3 - LONG SECTION

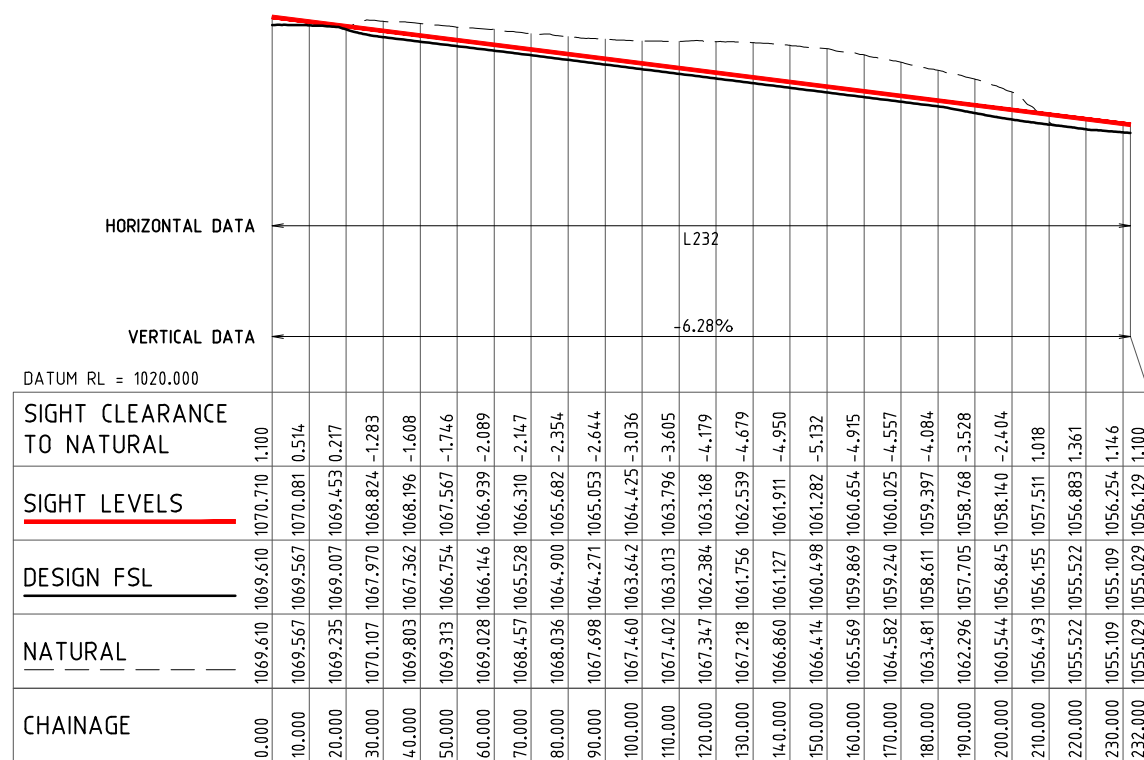


WEST SIGHT - LONGSECTION

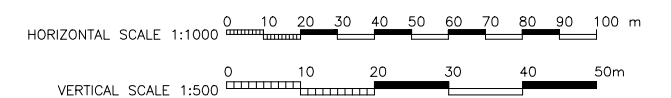
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DESIGNED:	A.M.	TITLE BORG RUNNING CREEK - RAZORBACK RD INTERSECTION LONGSECTION SHEET 2 OF 2			
DRAWN:	A.M.	SCALE	SIZE	SKETCH No.	REVISION
APPROVED:		AS SHOWN	B1	SK4153-012	0
		DATE		3-01-22	



WEST SIGHT - LONGSECTION



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DESIGNED:	A.M.	TITLE BORG RUNNING CREEK - RAZORBACK RD INTERSECTION EARTHWORKS WEST SIGHT LINE - LONGSECTION				REVISION	DATE
DRAWN:	B.S.	SCALE	SIZE	SKETCH No.	0	17-01-22	
APPROVED:	-	AS SHOWN	A1	SK4153-015			
		PC FILE: SK4153-015-0.DWG		PLOTING SCALE: 1:1		JN: 4153	

CENTRE LINE DATA
E = 768677.018
N = 6342911.541
RL = 1064.150
DATUM RL 1058.000

DESIGN		1065.614	1065.614
EXISTING		1065.833	1065.833
OFFSET	-20.000	1065.614	1065.614
	-15.000	1065.833	1065.833
	-10.000	1064.801	1064.801
	-5.000	1064.570	1064.570
	0.000	1064.150	1064.150
	5.000	1063.714	1063.714
	10.000	1066.265	1066.265
	15.000	1067.117	1067.117
	20.000	1067.249	1067.249

CH 380

CENTRE LINE DATA
E = 768721.323
N = 6342845.183
RL = 1068.660
DATUM RL 1062.500

DESIGN		1068.850	1068.850
EXISTING		1069.775	1069.775
OFFSET	-20.000	1068.850	1068.850
	-15.000	1069.775	1069.775
	-10.000	1068.838	1068.838
	-5.000	1068.873	1068.873
	0.000	1068.660	1068.660
	5.000	1068.294	1068.294
	10.000	1069.206	1069.206
	15.000	1069.196	1069.196
	20.000	1069.225	1069.225

CH 460

CENTRE LINE DATA
E = 768758.232
N = 6342774.209
RL = 1069.647
DATUM RL 1063.500

DESIGN		1069.237	1069.237
EXISTING		1069.220	1069.220
OFFSET	-20.000	1069.237	1069.237
	-15.000	1069.220	1069.220
	-10.000	1069.509	1069.509
	-5.000	1069.690	1069.690
	0.000	1069.647	1069.647
	5.000	1069.448	1069.448
	10.000	1069.342	1069.342
	15.000	1069.986	1069.986
	20.000	1070.283	1070.283

CH 540

CENTRE LINE DATA
E = 768798.025
N = 6342704.883
RL = 1069.263
DATUM RL 1061.500

DESIGN		1067.209	1067.209
EXISTING		1067.870	1067.870
OFFSET	-20.000	1067.209	1067.209
	-15.000	1067.870	1067.870
	-10.000	1068.228	1068.228
	-5.000	1069.053	1069.053
	0.000	1069.263	1069.263
	5.000	1069.303	1069.303
	10.000	1069.201	1069.201
	15.000	1068.918	1068.918
	20.000	1068.942	1068.942

CH 620

CENTRE LINE DATA
E = 768667.894
N = 6342925.692
RL = 1062.607
DATUM RL 1056.500

DESIGN		1064.850	1064.850
EXISTING		1065.062	1065.062
OFFSET	-20.000	1064.850	1064.850
	-15.000	1065.062	1065.062
	-10.000	1063.374	1063.374
	-5.000	1063.061	1063.061
	0.000	1062.607	1062.607
	5.000	1062.189	1062.189
	10.000	1064.808	1064.808
	15.000	1066.598	1066.598
	20.000	1066.707	1066.707

CH 360

CENTRE LINE DATA
E = 768711.816
N = 6342862.776
RL = 1067.839
DATUM RL 1061.500

DESIGN		1067.880	1067.880
EXISTING		1068.459	1068.459
OFFSET	-20.000	1067.880	1067.880
	-15.000	1068.459	1068.459
	-10.000	1068.236	1068.236
	-5.000	1068.139	1068.139
	0.000	1067.839	1067.839
	5.000	1067.431	1067.431
	10.000	1068.365	1068.365
	15.000	1068.233	1068.233
	20.000	1068.327	1068.327

CH 440

CENTRE LINE DATA
E = 768749.087
N = 6342791.996
RL = 1069.682
DATUM RL 1063.500

DESIGN		1069.865	1069.865
EXISTING		1069.897	1069.897
OFFSET	-20.000	1069.865	1069.865
	-15.000	1069.897	1069.897
	-10.000	1069.627	1069.627
	-5.000	1069.742	1069.742
	0.000	1069.682	1069.682
	5.000	1069.587	1069.587
	10.000	1069.616	1069.616
	15.000	1069.590	1069.590
	20.000	1070.073	1070.073

CH 520

CENTRE LINE DATA
E = 768787.130
N = 6342721.655
RL = 1069.378
DATUM RL 1061.000

DESIGN		1066.857	1066.857
EXISTING		1067.182	1067.182
OFFSET	-20.000	1066.857	1066.857
	-15.000	1067.182	1067.182
	-10.000	1068.638	1068.638
	-5.000	1069.264	1069.264
	0.000	1069.378	1069.378
	5.000	1069.288	1069.288
	10.000	1068.520	1068.520
	15.000	1069.387	1069.387
	20.000	1069.987	1069.987

CH 600

CENTRE LINE DATA
E = 768647.618
N = 6342938.585
RL = 1061.007
DATUM RL 1055.000

DESIGN		1063.715	1063.715
EXISTING		1063.945	1063.945
OFFSET	-20.000	1063.715	1063.715
	-15.000	1063.945	1063.945
	-10.000	1061.460	1061.460
	-5.000	1061.371	1061.371
	0.000	1061.007	1061.007
	5.000	1060.629	1060.629
	10.000	1062.996	1062.996
	15.000	1065.100	1065.100
	20.000	1065.348	1065.348

CH 340

CENTRE LINE DATA
E = 768701.101
N = 6342879.653
RL = 1066.812
DATUM RL 1060.500

DESIGN		1066.638	1066.638
EXISTING		1067.483	1067.483
OFFSET	-20.000	1066.638	1066.638
	-15.000	1067.483	1067.483
	-10.000	1067.331	1067.331
	-5.000	1067.162	1067.162
	0.000	1066.812	1066.812
	5.000	1066.374	1066.374
	10.000	1067.460	1067.460
	15.000	1067.440	1067.440
	20.000	1067.552	1067.552

CH 420

CENTRE LINE DATA
E = 768739.999
N = 6342809.811
RL = 1069.595
DATUM RL 1063.500

DESIGN		1069.784	1069.784
EXISTING		1069.533	1069.533
OFFSET	-20.000	1069.784	1069.784
	-15.000	1069.533	1069.533
	-10.000	1069.524	1069.524
	-5.000	1069.704	1069.704
	0.000	1069.595	1069.595
	5.000	1069.500	1069.500
	10.000	1069.537	1069.537
	15.000	1069.496	1069.496
	20.000	1069.950	1069.950

CH 500

CENTRE LINE DATA
E = 768776.924
N = 6342738.847
RL = 1069.481
DATUM RL 1061.500

DESIGN		1067.335	1067.335
EXISTING		1067.699	1067.699
OFFSET	-20.000	1067.335	1067.335
	-15.000	1067.699	1067.699
	-10.000	1069.152	1069.152
	-5.000	1069.439	1069.439
	0.000	1069.481	1069.481
	5.000	1069.318	1069.318
	10.000	1067.585	1067.585
	15.000	1069.061	1069.061
	20.000	1069.824	1069.824

CH 580

CENTRE LINE DATA
E = 768631.178
N = 6342949.963
RL = 1059.329
DATUM RL 1053.000

DESIGN		1061.790	1061.790
EXISTING		1062.046	1062.046
OFFSET	-20.000	1061.790	1061.790
	-15.000	1062.046	1062.046
	-10.000	1059.395	1059.395
	-5.000	1059.651	1059.651
	0.000	1059.329	1059.329
	5.000	1059.005	1059.005
	10.000	1060.675	1060.675
	15.000	1063.285	1063.285
	20.000	1063.410	1063.410

CH 320

CENTRE LINE DATA
E = 768689.631
N = 6342896.037
RL = 1065.583
DATUM RL 1059.500

DESIGN		1066.299	1066.299
EXISTING		1066.266	1066.266
OFFSET	-20.000	1066.299	1066.299
	-15.000	1066.266	1066.266
	-10.000	1066.184	1066.184
	-5.000	1065.949	1065.949
	0.000	1065.583	1065.583
	5.000	1065.091	1065.091
	10.000	1067.176	1067.176
	15.000	1067.236	1067.236
	20.000	1067.331	1067.331

CH 400

CENTRE LINE DATA
E = 768730.716
N = 6342827.526
RL = 1069.227
DATUM RL 1063.000

DESIGN		1069.781	1069.781
EXISTING		1069.622	1069.622
OFFSET	-20.000	1069.781	1069.781
	-15.000	1069.622	1069.622
	-10.000	1069.229	1069.229
	-5.000	1069.361	1069.361
	0.000	1069.227	1069.227
	5.000	1068.947	1068.947
	10.000	1070.078	1070.078
	15.000	1070.035	1070.035
	20.000	1070.052	1070.052

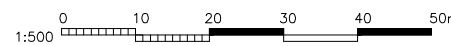
CH 480

CENTRE LINE DATA
E = 768767.445
N = 6342756.457
RL = 1069.546
DATUM RL 1062.500

DESIGN		1068.473	1068.473
EXISTING		1068.857	1068.857
OFFSET	-20.000	1068.473	1068.473
	-15.000	1068.857	1068.857
	-10.000	1069.286	1069.286
	-5.000	1069.577	1069.577
	0.000	1069.546	1069.546
	5.000	1069.401	1069.401
	10.000	1068.509	1068.509
	15.000	1069.673	1069.673
	20.000	1070.271	1070.271

CH 560

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53 BONVILLE AVENUE
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ABN: 54 003 887 374
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Fax: (02) 4964 2104
Email: arkill@arkill.com.au

DESIGNED: A.M.
DRAWN: A.M.
APPROVED: -

TITLE
**BORG RUNNING CREEK - RAZORBACK RD INTERSECTION
SECTIONS - SHEET 2 OF 3**

SCALE AS SHOWN SIZE A1 SKETCH No. REVISION DATE
0 13-01-22

CENTRE LINE DATA
 E = 768812.831
 N = 6342638.674
 RL = 1068.954
 DATUM RL 1062.000

DESIGN	-20.000	1067.852	1067.852																	
EXISTING		1068.117	1068.117																	
OFFSET				-10.000	1068.279	1068.279														

CH 700

CENTRE LINE DATA
 E = 768902.431
 N = 6342585.487
 RL = 1069.146
 DATUM RL 1061.500

DESIGN	-20.000	1067.116	1067.116																	
EXISTING		1067.370	1067.370																	
OFFSET				-10.000	1067.851	1067.851														

CH 780

CENTRE LINE DATA
 E = 768830.711
 N = 6342654.567
 RL = 1068.917
 DATUM RL 1061.500

DESIGN	-20.000	1067.340	1067.340																	
EXISTING		1067.956	1067.956																	
OFFSET				-10.000	1068.352	1068.352														

CH 680

CENTRE LINE DATA
 E = 768886.963
 N = 6342598.164
 RL = 1069.003
 DATUM RL 1061.000

DESIGN	-20.000	1066.618	1066.618																	
EXISTING		1067.002	1067.002																	
OFFSET				-10.000	1067.510	1067.510														

CH 760

CENTRE LINE DATA
 E = 768819.815
 N = 6342671.339
 RL = 1069.025
 DATUM RL 1062.000

DESIGN	-20.000	1068.243	1068.243																	
EXISTING		1068.928	1068.928																	
OFFSET				-10.000	1068.703	1068.703														

CH 660

CENTRE LINE DATA
 E = 768871.586
 N = 6342610.951
 RL = 1068.913
 DATUM RL 1061.000

DESIGN	-20.000	1066.776	1066.776																	
EXISTING		1066.701	1066.701																	
OFFSET				-10.000	1067.357	1067.357														

CH 740

CENTRE LINE DATA
 E = 768933.368
 N = 6342560.131
 RL = 1069.690
 DATUM RL 1063.000

DESIGN	-20.000	1068.988	1068.988																	
EXISTING		1069.072	1069.072																	
OFFSET				-10.000	1069.577	1069.577														

CH 820

CENTRE LINE DATA
 E = 768808.920
 N = 6342688.111
 RL = 1069.194
 DATUM RL 1062.500

DESIGN	-20.000	1068.326	1068.326																	
EXISTING		1068.339	1068.339																	
OFFSET				-10.000	1068.433	1068.433														

CH 640

CENTRE LINE DATA
 E = 768856.675
 N = 6342624.270
 RL = 1068.959
 DATUM RL 1061.000

DESIGN	-20.000	1067.133	1067.133																	
EXISTING		1067.504	1067.504																	
OFFSET				-10.000	1067.754	1067.754														

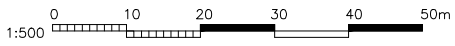
CH 720

CENTRE LINE DATA
 E = 768917.900
 N = 6342572.809
 RL = 1069.359
 DATUM RL 1062.500

DESIGN	-20.000	1068.315	1068.315																	
EXISTING		1068.215	1068.215																	
OFFSET				-10.000	1068.507	1068.507														

CH 800

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ARKHILL ENGINEERS		53 BONVILLE AVENUE THORNTON NSW 2322 PO BOX 29 MANTLAND NSW 2320		AR: 54 003 887 374 Phone: (02) 4088 0700 FAX: (02) 4964 2104 Email: arkhill@arkhill.com.au	
TITLE BORG RUNNING CREEK - RAZORBACK RD INTERSECTION SECTIONS - SHEET 3 OF 3					
DESIGNED:	A.M.	SCALE	AS SHOWN	SIZE	A1
DRAWN:	A.M.	SKETCH No.		REVISION	DATE
APPROVED:	-			0	13-01-22
PROJECT No. SK4153-023			PLOTING SCALE: 1:1		

CENTRE LINE DATA
 E = 768378.264
 N = 6342996.876
 RL = 1036.574
 DATUM RL 1027.500

DESIGN FSL	EXISTING	OFFSET
1033.603	1033.603	-20.000
1033.990	1033.990	-15.000
1035.762	1035.762	-10.000
1036.251	1036.251	-5.000
1036.574	1036.574	0.000
1036.824	1036.824	5.000
1036.544	1036.544	10.000
1038.411	1038.411	15.000
1039.212	1039.212	20.000

CH 60

CENTRE LINE DATA
 E = 768057.849
 N = 6342989.058
 RL = 1043.553
 DATUM RL 1037.000

DESIGN FSL	EXISTING	OFFSET
1042.761	1042.761	-20.000
1043.015	1043.015	-15.000
1043.270	1043.270	-10.000
1043.489	1043.489	-5.000
1043.553	1043.553	0.000
1043.514	1043.514	5.000
1043.138	1043.138	10.000
1043.255	1043.255	15.000
1046.309	1046.309	20.000

CH 140

CENTRE LINE DATA
 E = 768537.593
 N = 6342982.743
 RL = 1050.609
 DATUM RL 1041.000

DESIGN FSL	EXISTING	OFFSET
1046.919	1046.919	-20.000
1047.926	1047.926	-15.000
1050.704	1050.704	-10.000
1050.736	1050.736	-5.000
1050.609	1050.609	0.000
1050.435	1050.435	5.000
1050.310	1050.310	10.000
1049.719	1049.719	15.000
1050.900	1050.900	20.000

CH 220

CENTRE LINE DATA
 E = 768613.798
 N = 6342959.846
 RL = 1057.582
 DATUM RL 1051.000

DESIGN FSL	EXISTING	OFFSET
1059.332	1059.332	-20.000
1059.498	1059.498	-15.000
1057.528	1057.528	-10.000
1057.899	1057.899	-5.000
1057.582	1057.582	0.000
1057.202	1057.202	5.000
1057.823	1057.823	10.000
1060.518	1060.518	15.000
1060.713	1060.713	20.000
1060.509	1060.509	25.000
1061.038	1061.038	30.000
1061.118	1061.118	35.000
1061.116	1061.116	40.000
1061.281	1061.281	45.000

CH 300

CENTRE LINE DATA
 E = 768358.761
 N = 6343001.265
 RL = 1034.792
 DATUM RL 1024.500

DESIGN FSL	EXISTING	OFFSET
1030.390	1030.390	-20.000
1031.750	1031.750	-15.000
1033.876	1033.876	-10.000
1034.440	1034.440	-5.000
1034.792	1034.792	0.000
1035.020	1035.020	5.000
1034.937	1034.937	10.000
1036.439	1036.439	15.000
1039.223	1039.223	20.000

CH 40

CENTRE LINE DATA
 E = 768437.903
 N = 6342990.529
 RL = 1041.831
 DATUM RL 1035.000

DESIGN FSL	EXISTING	OFFSET
1040.913	1040.913	-20.000
1041.204	1041.204	-15.000
1041.333	1041.333	-10.000
1041.710	1041.710	-5.000
1041.831	1041.831	0.000
1041.648	1041.648	5.000
1041.370	1041.370	10.000
1041.812	1041.812	15.000
1045.428	1045.428	20.000

CH 120

CENTRE LINE DATA
 E = 768517.688
 N = 6342984.676
 RL = 1048.819
 DATUM RL 1040.000

DESIGN FSL	EXISTING	OFFSET
1046.228	1046.228	-20.000
1047.069	1047.069	-15.000
1048.690	1048.690	-10.000
1048.978	1048.978	-5.000
1048.819	1048.819	0.000
1048.643	1048.643	5.000
1049.016	1049.016	10.000
1048.782	1048.782	15.000
1050.098	1050.098	20.000

CH 200

CENTRE LINE DATA
 E = 768595.609
 N = 6342976.113
 RL = 1055.828
 DATUM RL 1049.000

DESIGN FSL	EXISTING	OFFSET
1056.305	1056.305	-20.000
1056.281	1056.281	-15.000
1055.952	1055.952	-10.000
1056.136	1056.136	-5.000
1055.828	1055.828	0.000
1055.436	1055.436	5.000
1055.403	1055.403	10.000
1058.043	1058.043	15.000
1058.497	1058.497	20.000
1058.665	1058.665	25.000
1058.906	1058.906	30.000
1058.951	1058.951	35.000
1059.239	1059.239	40.000
1059.224	1059.224	45.000

CH 280

CENTRE LINE DATA
 E = 768339.769
 N = 6343007.507
 RL = 1033.004
 DATUM RL 1022.500

DESIGN FSL	EXISTING	OFFSET
1028.528	1028.528	-20.000
1029.389	1029.389	-15.000
1032.242	1032.242	-10.000
1032.686	1032.686	-5.000
1033.004	1033.004	0.000
1033.279	1033.279	5.000
1033.410	1033.410	10.000
1036.054	1036.054	15.000
1038.836	1038.836	20.000

CH 20

CENTRE LINE DATA
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 N = 6342992.111
 RL = 1040.049
 DATUM RL 1033.500

DESIGN FSL	EXISTING	OFFSET
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1039.527	1039.527	-15.000
1039.554	1039.554	-10.000
1039.868	1039.868	-5.000
1040.049	1040.049	0.000
1040.025	1040.025	5.000
1039.782	1039.782	10.000
1040.305	1040.305	15.000
1045.534	1045.534	20.000

CH 100

CENTRE LINE DATA
 E = 768497.742
 N = 6342986.138
 RL = 1046.981
 DATUM RL 1039.000

DESIGN FSL	EXISTING	OFFSET
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1045.889	1045.889	-15.000
1046.752	1046.752	-10.000
1047.079	1047.079	-5.000
1046.981	1046.981	0.000
1046.769	1046.769	5.000
1047.161	1047.161	10.000
1047.523	1047.523	15.000
1048.493	1048.493	20.000

CH 180

CENTRE LINE DATA
 E = 768576.704
 N = 6342976.620
 RL = 1054.120
 DATUM RL 1047.500

DESIGN FSL	EXISTING	OFFSET
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1053.822	1053.822	-15.000
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1054.451	1054.451	-5.000
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1053.771	1053.771	5.000
1053.825	1053.825	10.000
1055.300	1055.300	15.000
1055.580	1055.580	20.000
1055.872	1055.872	25.000
1056.078	1056.078	30.000
1056.253	1056.253	35.000
1056.560	1056.560	40.000
1056.681	1056.681	45.000

CH 260

CENTRE LINE DATA
 E = 768321.524
 N = 6343015.683
 RL = 1031.202
 DATUM RL 1021.000

DESIGN FSL	EXISTING	OFFSET
1027.155	1027.155	-20.000
1028.045	1028.045	-15.000
1030.347	1030.347	-10.000
1030.863	1030.863	-5.000
1031.202	1031.202	0.000
1031.520	1031.520	5.000
1031.697	1031.697	10.000
1034.570	1034.570	15.000
1036.623	1036.623	20.000

CH 0

CENTRE LINE DATA
 E = 768398.062
 N = 6342994.057
 RL = 1038.316
 DATUM RL 1031.500

DESIGN FSL	EXISTING	OFFSET
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1037.587	1037.587	-15.000
1037.714	1037.714	-10.000
1038.110	1038.110	-5.000
1038.316	1038.316	0.000
1038.459	1038.459	5.000
1038.293	1038.293	10.000
1039.564	1039.564	15.000
1043.212	1043.212	20.000

CH 80

CENTRE LINE DATA
 E = 768477.796
 N = 6342987.608
 RL = 1045.254
 DATUM RL 1038.000

DESIGN FSL	EXISTING	OFFSET
1044.112	1044.112	-20.000
1044.490	1044.490	-15.000
1045.014	1045.014	-10.000
1045.292	1045.292	-5.000
1045.254	1045.254	0.000
1045.070	1045.070	5.000
1045.224	1045.224	10.000
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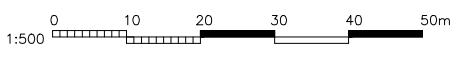
CH 160

CENTRE LINE DATA
 E = 768557.333
 N = 6342979.571
 RL = 1052.390
 DATUM RL 1044.500

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1050.483	1050.483	-15.000
1052.222	1052.222	-10.000
1052.583	1052.583	-5.000
1052.390	1052.390	0.000
1052.051	1052.051	5.000
1051.865	1051.865	10.000
1051.873	1051.873	15.000
1052.096	1052.096	20.000
1052.886	1052.886	25.000
1053.701	1053.701	30.000
1054.139	1054.139	35.000
1054.468	1054.468	40.000
1054.626	1054.626	45.000

CH 240

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TITLE: **BORG RUNNING CREEK - RAZORBACK RD INTERSECTION
 EARTHWORKS WEST SIGHT LINE - SECTIONS - SHEET 1 OF 3**

DESIGNED:	A.M.
DRAWN:	B.S.
APPROVED:	-

SCALE	SIZE	SKETCH No.	REVISION	DATE
AS SHOWN	A1		0	17-01-22

PC FILE: SK4153-025-0.DWG PLOTTING SCALE: 11 JN: 4153

CENTRE LINE DATA
 E = 768677.018
 N = 6342911.561
 RL = 1064.150
 DATUM RL 1056.000

DESIGN FSL	EXISTING	OFFSET
1065.614	1065.614	-20.000
1065.833	1065.833	-15.000
1064.801	1064.801	-10.000
1064.570	1064.570	-5.000
1064.150	1064.150	0.000
1063.714	1063.714	5.000
1063.325	1066.265	10.000
1062.945	1067.117	15.000
1062.565	1067.249	20.000
1062.185	1067.278	25.000
1062.155	1067.293	30.000
1063.826	1067.319	35.000
1065.497	1067.124	40.000
1067.061	1067.061	45.000

CH 380

CENTRE LINE DATA
 E = 768721.323
 N = 6342845.183
 RL = 1068.660
 DATUM RL 1061.000

DESIGN FSL	EXISTING	OFFSET
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1069.775	1069.775	-15.000
1068.838	1068.838	-10.000
1068.873	1068.873	-5.000
1068.660	1068.660	0.000
1068.294	1068.294	5.000
1067.655	1069.206	10.000
1067.017	1069.196	15.000
1066.766	1069.225	20.000
1068.320	1069.390	25.000
1069.266	1069.266	30.000
1069.271	1069.271	35.000
1069.186	1069.186	40.000
1069.231	1069.231	45.000

CH 460

CENTRE LINE DATA
 E = 768758.232
 N = 6342774.289
 RL = 1069.647
 DATUM RL 1063.500

DESIGN FSL	EXISTING	OFFSET
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1069.220	1069.220	-15.000
1069.509	1069.509	-10.000
1069.690	1069.690	-5.000
1069.647	1069.647	0.000
1069.448	1069.448	5.000
1069.342	1069.342	10.000
1069.986	1069.986	15.000
1070.283	1070.283	20.000

CH 540

CENTRE LINE DATA
 E = 768662.894
 N = 6342925.692
 RL = 1062.607
 DATUM RL 1055.000

DESIGN FSL	EXISTING	OFFSET
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1065.062	1065.062	-15.000
1063.374	1063.374	-10.000
1063.061	1063.061	-5.000
1062.607	1062.607	0.000
1062.189	1062.189	5.000
1061.866	1064.808	10.000
1061.554	1066.598	15.000
1061.242	1066.707	20.000
1060.930	1066.634	25.000
1061.426	1066.617	30.000
1063.114	1066.484	35.000
1064.801	1066.278	40.000
1066.080	1066.080	45.000

CH 360

CENTRE LINE DATA
 E = 768711.816
 N = 6342862.776
 RL = 1067.839
 DATUM RL 1059.500

DESIGN FSL	EXISTING	OFFSET
1067.880	1067.880	-20.000
1068.459	1068.459	-15.000
1068.236	1068.236	-10.000
1068.139	1068.139	-5.000
1067.839	1067.839	0.000
1067.431	1067.431	5.000
1066.870	1068.365	10.000
1066.292	1068.233	15.000
1065.716	1068.327	20.000
1065.813	1068.348	25.000
1067.400	1068.409	30.000
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1068.400	1068.400	40.000
1068.383	1068.383	45.000

CH 440

CENTRE LINE DATA
 E = 768749.087
 N = 6342791.996
 RL = 1069.682
 DATUM RL 1063.500

DESIGN FSL	EXISTING	OFFSET
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1069.627	1069.627	-10.000
1069.742	1069.742	-5.000
1069.682	1069.682	0.000
1069.587	1069.587	5.000
1069.616	1069.616	10.000
1069.590	1069.590	15.000
1070.073	1070.073	20.000

CH 520

CENTRE LINE DATA
 E = 768647.618
 N = 6342938.585
 RL = 1061.007
 DATUM RL 1054.000

DESIGN FSL	EXISTING	OFFSET
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1063.945	1063.945	-15.000
1061.460	1061.460	-10.000
1061.371	1061.371	-5.000
1061.007	1061.007	0.000
1060.629	1060.629	5.000
1060.403	1062.996	10.000
1060.153	1065.100	15.000
1059.907	1065.348	20.000
1059.694	1065.243	25.000
1061.227	1065.192	30.000
1062.923	1065.089	35.000
1064.620	1064.953	40.000
1064.916	1064.916	45.000

CH 340

CENTRE LINE DATA
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 N = 6342879.653
 RL = 1066.812
 DATUM RL 1058.500

DESIGN FSL	EXISTING	OFFSET
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1067.331	1067.331	-10.000
1067.162	1067.162	-5.000
1066.812	1066.812	0.000
1066.374	1066.374	5.000
1065.854	1067.460	10.000
1065.334	1067.440	15.000
1064.813	1067.552	20.000
1064.293	1067.692	25.000
1065.342	1067.754	30.000
1066.964	1067.916	35.000
1067.879	1067.879	40.000
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CH 420

CENTRE LINE DATA
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 N = 6342809.811
 RL = 1069.595
 DATUM RL 1063.500

DESIGN FSL	EXISTING	OFFSET
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1069.533	1069.533	-15.000
1069.524	1069.524	-10.000
1069.704	1069.704	-5.000
1069.595	1069.595	0.000
1069.500	1069.500	5.000
1069.537	1069.537	10.000
1069.496	1069.496	15.000
1069.906	1069.906	20.000
1070.406	1070.406	25.000
1070.407	1070.407	30.000
1070.431	1070.431	35.000
1070.564	1070.564	40.000
1070.455	1070.455	45.000

CH 500

CENTRE LINE DATA
 E = 768631.178
 N = 6342949.963
 RL = 1059.329
 DATUM RL 1053.000

DESIGN FSL	EXISTING	OFFSET
1061.790	1061.790	-20.000
1062.046	1062.046	-15.000
1059.395	1059.395	-10.000
1059.651	1059.651	-5.000
1059.329	1059.329	0.000
1059.005	1059.005	5.000
1058.829	1060.675	10.000
1058.705	1063.285	15.000
1058.570	1063.440	20.000
1059.871	1063.522	25.000
1061.555	1063.514	30.000
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1063.494	1063.494	45.000

CH 320

CENTRE LINE DATA
 E = 768689.631
 N = 6342896.037
 RL = 1065.583
 DATUM RL 1057.000

DESIGN FSL	EXISTING	OFFSET
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1066.266	1066.266	-15.000
1066.184	1066.184	-10.000
1065.949	1065.949	-5.000
1065.583	1065.583	0.000
1065.091	1065.091	5.000
1064.617	1067.176	10.000
1064.153	1067.236	15.000
1063.689	1067.331	20.000
1063.226	1067.389	25.000
1063.365	1067.487	30.000
1064.987	1067.514	35.000
1066.609	1067.387	40.000
1067.265	1067.265	45.000

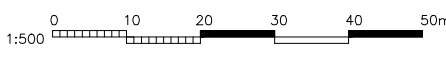
CH 400

CENTRE LINE DATA
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 N = 6342827.526
 RL = 1069.227
 DATUM RL 1062.000

DESIGN FSL	EXISTING	OFFSET
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1069.622	1069.622	-15.000
1069.229	1069.229	-10.000
1069.361	1069.361	-5.000
1069.227	1069.227	0.000
1068.947	1068.947	5.000
1068.312	1070.078	10.000
1067.910	1070.035	15.000
1069.460	1070.052	20.000
1070.037	1070.037	25.000
1070.005	1070.005	30.000
1069.962	1069.962	35.000
1069.917	1069.917	40.000
1069.946	1069.946	45.000

CH 480

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TITLE: **BORG RUNNING CREEK - RAZORBACK RD INTERSECTION
 EARTHWORKS WEST SIGHT LINE - SECTIONS - SHEET 2 OF 3**

DESIGNED: A.M.
 DRAWN: B.S.
 APPROVED: -

SCALE: AS SHOWN
 SIZE: A1
 SKETCH No.:
 REVISION: 0
 DATE: 17-01-22

PC FILE: SK4153-025-0.DWG
 PLOTTING SCALE: 1:1
 JN: 4153

Appendix C - Draft Operational Traffic Management Plan

David Pavey Pty Ltd trading as

Pavey Consulting Services

Specialising in

Traffic Impact Assessments and Transportation Planning
Road Safety, Traffic Management Plans and Traffic Control Plans
Civil and Structural Design
Project Management and Contract Administration
Mediation and Government Relations

Operational Traffic Management Plan

Quarry Running Stream

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1.0 INTRODUCTION

This document provides an Operational Traffic Management Plan (OTMP) proposed quarry at Running Stream NSW.

2.0 REFERENCES

- ◆ Work Health & Safety Act (NSW) 2011
- ◆ Work Health & Safety Regulations (NSW) 201
- ◆ Work Health & Safety (National Uniform Legislation) Act 2011
- ◆ Work Health & Safety (National Uniform Legislation) Regulations 2011
- ◆ Safe Work Australia: Construction Work - Code of Practice (2013)
- ◆ Safe Work Australia: General Guide for Workplace Traffic Management (2014)
- ◆ Safe Work Australia: Traffic Management: Guide for Construction Work (2014)

3.0 WAYS TO CONTROL TRAFFIC RISKS

Keeping people and vehicles apart

The best way to protect pedestrians is to make sure people and vehicles cannot interact. Where powered mobile plant is used at a workplace, you must ensure it does not collide with pedestrians or other powered mobile plant.

This can be achieved by not allowing vehicles in pedestrian spaces or not allowing pedestrians in vehicle operating areas, for example using overhead walkways.

However, this may not be reasonably practicable in all workplaces. If people and vehicles cannot be separated you should consider using:

- barriers or guardrails at building entrances and exits to stop pedestrians walking in front of vehicles,
- high impact traffic control barriers,
- temporary physical barriers, or
- separate, clearly marked footpaths or walkways e.g. using lines painted on the ground or different coloured surfacing.

Vehicle routes

Vehicle routes at the workplace should have a firm and even surface, be wide and high enough for the largest vehicle using them and be well maintained and free from obstructions. They should be clearly sign-posted to indicate speed limits, traffic calming measures like speed humps and parking areas.

Reducing speed is very important where administrative control measures are the only reasonably practicable approach. Speed limits should be implemented and enforced and traffic calming devices like speed humps considered.

Pedestrian crossings

Pedestrian crossings should be clearly marked with ground markings, lights or signs. If the vehicle route to be crossed is a road or railway consider control measures that will work with those already established by the relevant authority, for example a local council or rail authority.

Both pedestrians and vehicles should have good visibility, for example pallet goods should not be stored in a way that would obscure vision.

Procedures indicating who has right of way at crossings should also be established.

Parking areas

Parking may be needed for workers, visitors, trucks and other vehicles used in the workplace. Consider setting out the workplace so parking areas:

- are located away from busy work areas and traffic routes,
- have walkways leading to and from parking areas which are separated from vehicles or vehicle routes e.g. use physical controls like barriers or bollards to prevent vehicles from crossing into walking areas, and
- are clearly marked and sign-posted, well-lit and unobstructed.

Reversing vehicles

If reasonably practicable eliminate the need for reversing by using drive-through loading and unloading systems, multi-directional mobile plant or rotating cabins. Where this is not possible consider:

- using devices like reversing sensors, reversing cameras, mirrors, rotating lights or audible reversing alarms,
- using a person to direct the reversing vehicle if they cannot see clearly behind—this person should be in visible contact with the driver at all times and wear high-visibility clothing,
- providing designated clearly marked, signposted and well-lit reversing areas, and
- excluding non-essential workers from the area.

Loading and unloading vehicles

It is important to make sure visitors including visiting drivers are aware of the workplace layout, the route they should take and safe working procedures for the workplace. Provide drivers with safe access to amenities away from loading areas or other vehicular traffic.

Provide effective ways to warn of loading in progress to other plant operators, drivers and pedestrians. Warning devices can include signage, cones, lights, alarms and horns.

Signs and road markings

Clear road markings like reflective paint and signs should be used to alert pedestrians and vehicle operators to traffic hazards in the workplace.

Signs should be provided to indicate exclusion and safety zones, parking areas, speed limits, vehicle crossings and hazards like blind corners, steep gradients and where forklifts are in use.

Lighting

Traffic routes, manoeuvring areas and yards should be well lit with particular attention given to junctions, buildings, walkways and vehicles routes. Where possible they should be designed to avoid extreme light variation, for example drivers moving from bright into dull light or vice versa.

4.0 COUNCIL CONSULTATION

TBC after Council Consultation

5.0 SITE LOCATION

TBC after Development Approval has been obtained.

6.0 TRAFFIC MANAGEMENT PLAN

Introduction

The purpose of this document is to minimise the impacts of the heavy vehicle traffic on Davis Rd, the surrounding properties and on the community and to manage the movement of heavy vehicles using best industry practice.

Objectives

The objectives of this Traffic Management Plan and Driver Code of Conduct are to:

- a) Ensure compliance with the conditions,
- b) Encourage compliance and acceptance of the Truck Driver Code of Conduct by all heavy vehicle drivers,
- c) Minimise the heavy vehicle impacts on the community,
- d) Foster an understanding and awareness within the company of community expectations and legislative requirements in regard to heavy vehicle movements,
- e) Protect and enhance public safety through compliance with relevant road rules, and
- f) Increase OH&S understanding in relation to fatigue, vehicle operation in public areas and obligation to the general public.

Project Description

TBC after Development Approval has been obtained.

Site Access and Internal Operations

Access Arrangements

Access to the site is controlled by Borg Resources staff. A programmable swipe card/tag will be provided to all employees and regular contractors to activate boom gates and access the weighbridge.

The site speed limit is 15 km/h and this will be enforced.

Passenger Vehicles

The suitability of the proposed access driveway with respect to accommodating passenger vehicles is assessed based on guidelines provided within the Australian Standard for Off-Street Car parking (AS2890.1-2004). This publication provides driveway design recommendations based on several site characteristics such as the number and classification of vehicles to be accommodated on-site and the functional role of the frontage road.

It is evident that the proposed combined ingress/egress driveway suitably accords with the design criteria specified within AS2890.1-2004 and is therefore considered to be satisfactory in terms of servicing passenger vehicles.

Passenger Vehicles

Upon entry to the subject site, passenger vehicles will access the at-grade passenger vehicle parking areas.

The parking bays and internal circulation of the parking areas has been designed to accord with the relevant requirements of AS2890.1 and AS2890.6.

The above compliance with the relevant AS2890.1 and AS2890.6- specifications is anticipated to result in safe and efficient internal manoeuvring and parking space

accessibility.

Marked pedestrian paths are provided to guide pedestrians from carparks to reception and office locations.

Signage has been erected to direct all visitors to report to office prior to moving around the site.

Heavy Vehicles

Traffic movements for a range of heavy vehicles has been examined by preparing several swept path plans, which have been overlaid on the site.

This sweep analysis indicates that all heavy vehicles proposed to service the facility are capable of manoeuvring within the site in a safe and efficient manner without any unreasonable encroachment on internal passenger vehicle parking areas or structures. Accordingly, the internal heavy vehicle manoeuvring arrangements are satisfactory.

Hours of Operation

The current approved development is approved to process materials during the following hours:

- 7am to 5pm Monday to Friday,
- 8am to 1pm Saturdays, and
- No work on Sundays or Public Holidays.

Minimising Vehicle Movements

Traffic movement around the workplace should be minimised as much as possible. This will be achieved where practicable by:

- Controlling entry/exit to the work area by planning or engineering processes (e.g. gates, signage, speed control),
- Developing storage areas so delivery vehicles do not have to cross the site,
- Scheduling work processes to minimise the number of vehicles operating at the same time, and
- Scheduling work processes to minimise the number of vehicles operating while people are moving through an area (e.g. start and finish of shifts).

Haul Roads

TBC after Development Approval has been obtained.

Monitoring of Product Transport

The Proponent shall keep accurate records of:

- The amount of quarry products transported from the site (per calendar month and year),
- The number of laden vehicle movements from the site (per hour, day, week, calendar month and year), and
- Monitor complaints with respect to the usage of Davis Rd and other haul roads.

7.0 DRIVER CODE OF CONDUCT

A driver code of conduct has been developed for the site and is included in Appendix C.

This document includes:

Heavy vehicle drivers

- Have undertaken a site induction carried out by an approved member of staff,
- Hold a valid driver's licence for the class of vehicle that they operate,
- Operate the vehicle in a safe manner within and external to the site, and
- Comply with the direction of authorised site personnel when within the site.

Heavy Vehicle Speed

Heavy vehicle drivers need to comply with:

- signposted speed limits on haul routes,
- internally within the site, and
- Drivers and truck operators are to be aware of the "Three Strikes Scheme" introduced by the Roads and Maritime Services which applies to all vehicles over 4.5 tonnes. When a heavy vehicle is detected travelling at 15 km/h or more over the posted or relevant heavy vehicle speed limit by a mobile Police unit or fixed speed camera, the Roads and Maritime Services will record a strike against that vehicle. If three strikes are recorded within a three-year period, the Transport for NSW will act to suspend the registration of that vehicle (up to three months).

Heavy Vehicles Driver Fatigue

Fatigue is one of the biggest causes of accidents for heavy vehicle drivers. The Heavy Vehicle Driver Fatigue Reform was therefore developed by the National Transport Commission (NTC) and approved by Ministers from all States and Territories in February 2007.

The heavy vehicle driver fatigue law commenced in NSW on 28 September 2008 and applies to trucks and truck combinations over 12 tonne GVM.

Heavy Vehicle Compression Braking

Compression braking by heavy vehicles is a source of irritation to the community generating many complaints especially at night when residents are especially sensitive to noise.

In some instances, compression braking is required for safety reasons however when passing through or adjacent to residential areas or isolated farmsteads a reduction in the speed of the vehicle is recommended to reduce the instances and severity of compression braking.

Load Covering

Loose material on the road surface has the potential to cause road crashes and vehicle damage.

All trucks arriving at or departing the site whether loaded with material or not are required to have an effective cover over their load for the duration of the trip.

All care is to be taken to ensure that all loose debris from the vehicle body and wheels is removed prior to leaving the site.

Drivers must ensure that following tipping that the tailgate is locked before leaving the site.

Vehicle Departure and Arrival

Trucks should only be scheduled to arrive during operating hours to minimise the need for on street parking.

8.0 COMPLIANCE MONITORING

Commencement of Traffic Management Plan & Driver Code of Conduct

It is proposed that this Traffic Management Plan will be initiated when the project becomes operational and reviewed after 12 months of operation.

The Driver Code of Conduct is to be signed by individual drivers and authorised representative of Borg Resources at the time when drivers attend their site induction or shortly thereafter.

Monitoring Measures

A formal observation of compliance at three monthly intervals will be undertaken to document any remedial actions with employees, heavy vehicle drivers or haulage companies that may be necessary as a result of these observations.

DRAFT

Appendix D - Draft Driver Code of Conduct

Borg Resources Drivers Code of Conduct

This document sets out the truck driver requirements for all employees and contractors to Borg Resources.

DECLARATION

I, the undersigned, hereby agree to abide by Borg Resources' Driver Code of Conduct for the transportation of timber resources to/ from the sites in Oberon in a safe manner.

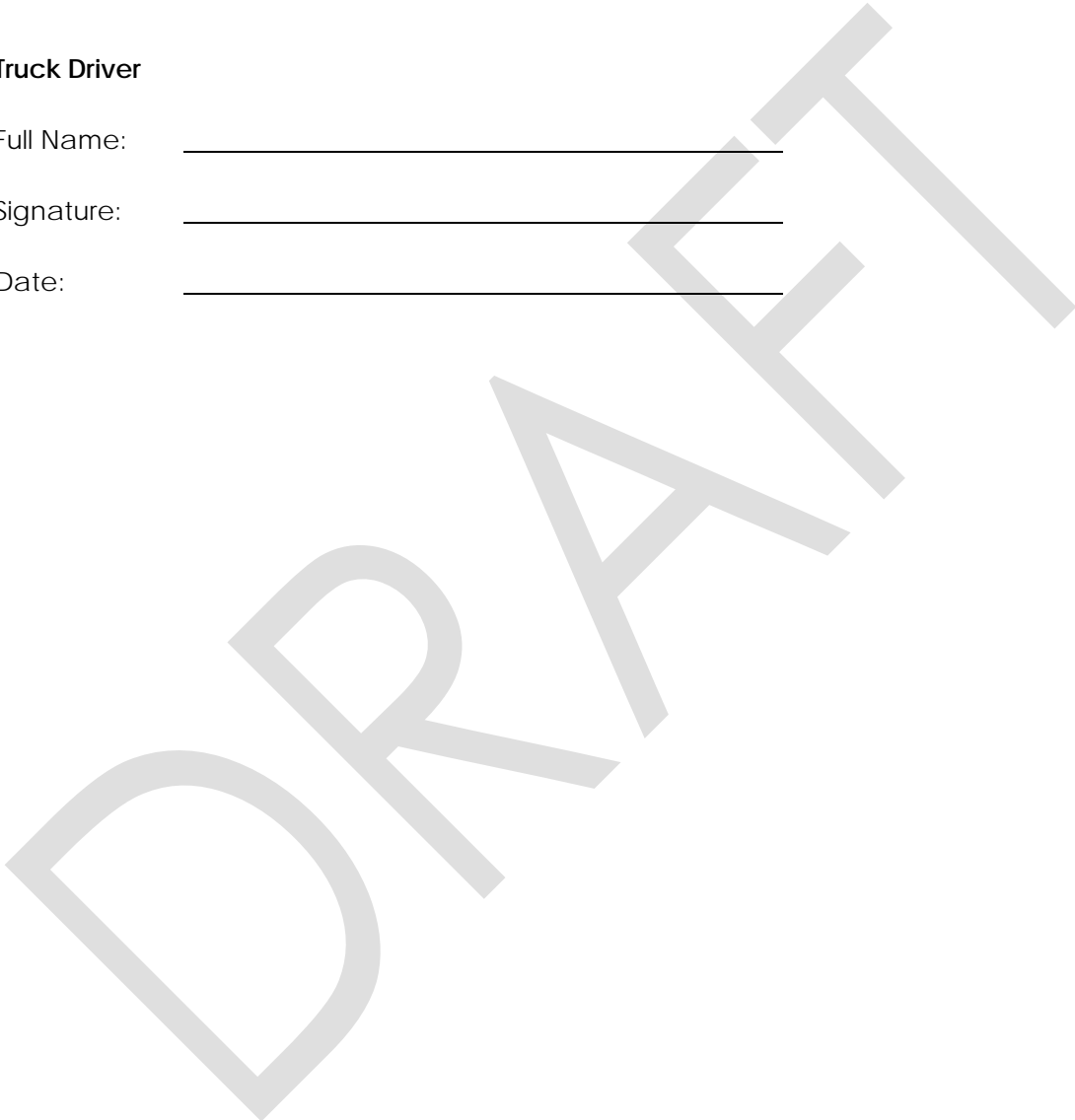
I have read and understand the requirements outlined in the Code and will, to the best of my ability, comply and assist with their implementation, requirements and ongoing compliance.

Truck Driver

Full Name: _____

Signature: _____

Date: _____



General Requirements

The Drivers Code of Conduct is distributed to all employee drivers and sub-contractors transporting quarry products to and from Running Stream Quarry.

The Code would be provided to each driver to read and sign to confirm they have understood and pledge to follow the haulage instructions.

Heavy vehicle drivers hauling to and from the subject site must:

- Have read and signed the Drivers Code of Conduct (this document) prior as a condition of their employment;
- Hold a valid driver's license for the class of vehicle that is being operated;
- Operate the vehicle in a safe manner while on site and public road network;
- Comply with the directions of Borg Resources supervision, safety and operational requirements and nominated Haulage Routes;
- All drivers are to use seat belts when driving;
- All drivers are to drive to the sign posted speed limit, both on public roads, private roads and within the site.

Site Access

Access to the site is controlled by Borg Resources staff. A programmable swipe card/tag will be provided to all employees and regular contractors to activate boom gates and access the weighbridge.

The site speed limit is 15 km/h and this will be enforced.



Heavy Vehicle Haul Routes

All heavy vehicle drivers must adhere to the designated truck routes to/from the site as follows:

- Approach routes:
 - Travel on Castlereagh Highway and Razorback Road in westerly direction, turn left into Running Stream Quarry.
 - Or as determined by the Haulage Plan as provided
- Departure routes:
 - Turn right onto Razorback Road and then proceed to Castlereagh Highway .
 - Or as determined by the Haulage Plan as provided

Heavy Vehicle Speed

Truck drivers must comply with the Australian Road Rules with travelling along public roads.

Drivers are to observe the posted speed limits and adjust speed appropriately to suit the road and weather conditions at the time.

Speed limits on route to the site can be between 40km/hr (school zones) up to 100km/hr. The maximum speed that a vehicle must travel is the signposted speed. Warning signs indicating a reduction in speed ahead must also be obeyed. These signs are shown below.

Typical NSW Road Speed Limit Signs



Speed Reduction Ahead Warning Sign



The speed limit within the site is 15km/hr (unless sign posted otherwise in an area) which is to be strictly maintained.

Heavy Vehicles Driver Fatigue

The heavy vehicle driver fatigue law commenced in NSW in 2008 and applies to trucks and truck combinations over 8 tonnes GVM (however, Ministerial Exemption Notices may apply).

Under the law, industry has the choice of operating under three fatigue management schemes, namely:

1. Standard Hours of Operation – Borg Resources limited to 13 hours per day
2. Basic Fatigue Management (BFM)
3. Advanced Fatigue management (AFM).

All heavy vehicle drivers associated with the Resources team are to be aware of their adopted fatigue management scheme and operate within its requirements.

Heavy Vehicle Compression Braking

Compression braking on route to or hauling away from site should only be used when required and for safety reasons. It is not to be used in areas where prohibited.

Heavy Vehicle Noise

Impulsive and Tonal noise generating activities shall not be undertaken at site outside normal operating hours.

Load Covering (where applicable)

All loaded trucks arriving at and departing from the site are required to have an effective cover over their load for the duration of the journey. The loadcover may be removed only upon arrival at the destination (ie. at the site).

Care must be taken to ensure that all loose debris from vehicles and wheels is removed prior to exiting the site.

Site management is to monitor loose material on the side of the haul route and take appropriate action regularly.

Other Safety Considerations along the Haul Route

Heavy vehicle drivers should be aware of the following:

- Load restraint remains the responsibility of every driver and failure to restrain a load will result in personal fines.
- Concealed driveways – drivers are to drive with caution around any signed concealed driveways
- Adverse weather safety – drivers should adjust their driving speed to suit weather conditions at the time. Be particularly aware of hazardous driving conditions for all road users in these conditions.
- Do not cross water courses when the water depth is above 100mm, report the situation to your manager to make alternative route arrangements.
- Remember, some of our trucks have signs on the rear trailer advising motorists of our safe driver practices, plus contact details and all have a registration number and your driving behaviour, good or poor, can be reported to Borg management.